Stormwater Management Report

Gratuity Brook Farm Estates Definitive Subdivision

> 63 Gratuity Road Groton, MA

July 3, 2024 Revised: March 10, 2025

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# **Project Narrative**

The site known as 63 Gratuity Road, Groton, MA is approximately 52.8 acres and consists mostly of woodland, with a small portion of the property being cleared near Gratuity Road. The lot is shown on the Town of Groton Tax Map 216 as Parcel 47 and is located entirely in the Residential/Agricultural Zoning District.

The proposed project involves the construction of a 22 unit (12 duplex buildings) residential development which will be accessed from two dead end roads, along with associated grading, utility installation, and stormwater management system. The stormwater management system for the proposed development has been designed in accordance with the DEP Stormwater Management Policy to the maximum extent practicable. The combination of Best Management Practices (BMPs) provided would achieve the required removal of total suspended solids/phosphorus as required by the DEP Storm Water Management Policy and the local Stormwater Management Regulations.

The development would incorporate a combination of open and closed drainage systems designed to handle runoff that would be generated by the proposed development. The drainage system would collect runoff which has been designed to treat the runoff for the 2-year, 10-year, 25-year, and 100-year storm events. The proposed grading would be in such a manner as to not increase the post development peak rate of runoff to the adjacent properties.

### 1.1 TOPOGRAPHY, GEOLOGY AND SOILS

The property lies on the east side of the Nashua River, bounded by Gratuity Road to the north, Jenkins Road to the south, residential properties to the east, and Hazel Grove Park and fairgrounds to the west. The property is bisected by an intermittent stream which flows east to west towards Nashua River. The topography of the site consists of gently sloping grades that generally slope north from land south of the stream, and south from land north of the stream. The high point north of the stream is found near Gratuity Road, at the location of a razed dwelling at an elevation of approximately 216', and at southeast corner adjacent to Jenkins Road at elevation 215'. The lowest point found is at the intermittent stream, at elevation 208' (top of stream bank).

Vegetation onsite is primarily wooded throughout the site with a mix of deciduous and coniferous trees. Some cleared grassy areas and pavement exist at disturbed arounds that surround the location of the razed dwelling.

The property is bisected by an intermittent stream, with several small areas bordering vegetated wetlands adjacent to the stream. The wetlands were flagged by Creative Land & Water Engineering, LLC. and field located by S. J. Mullaney Engineering, Inc. and confirmed in and



Order of Resource Area Delineation by the Town of Groton Conservation Commission on November 12, 2019 (MassDEP File Number 169-1190). An extension permit was issued by the Groton Conservation Commission for the ORAD on January 23, 2024.

The Natural Resources Conservation Service (NRCS) Soil survey of Middlesex County, Massachusetts has mapped the soils on the project site. The soil consists of Hinckley Loamy Sand, Winsor loamy sand, and Deerfield loamy sand. Soil testing performed on the site confirms the mapping of the soils, with well drained sandy soils found throughout all test holes performed.



# 2.0 Standard 1: No New Untreated Discharges

The MA Stormwater Handbook requires that the project demonstrates that there are no new untreated discharges and that new discharges will not cause erosion or scour downstream wetlands. The proposed project will not result in any new untreated discharges. New erosion and sediment control devices will be put in place to ensure there will be no erosion or scour to downstream properties or wetlands.



# 3.0 Standard 2: Peak Rate Attenuation

Standard 2 requires that peak rates of flow be attenuated for the proposed condition. A full hydrologic analysis of the pre-development and post-development conditions was completed. A more detailed explanation of the existing and proposed peak rates of runoff is included below. The following section outlines the procedure for determining the peak rates for the existing condition as well as the methods for attenuating the peak flows in the proposed condition.

#### 3.1 EXISTING CONDITIONS

There are two drainage areas that discharge to different points around the project site or to the wetlands within the site (see Figure 2 – Pre-Development Drainage Map). The following assumptions were made for the purpose of this hydrologic analysis:

• Whenever possible, the property line, flagged wetland line, and/or an arbitrary line, outside the limit of proposed work was delineated as the watershed boundary.

Brief descriptions of each contributing area are below (see Table 1 for area acreage for each drainage area):

#### 3.1.1 Existing Drainage Areas

**Existing Drainage Area EX-1 (A&B)** consists of wooded areas both south (EX-1A) and north (EX-1B) of the intermittent stream at the center of the property, as well as a portion of the grass and pavement near the razed dwelling at Gratuity Road. The drainage area slopes towards the intermittent stream (DP-1). Runoff from this area flows overland and is undetained and untreated. The discharge point is designated as DP-1 on the existing HydroCAD analysis and accompanying drainage map.

**Existing Drainage Area EX-2** consists of a portion of the remaining grass area and pavement at the razed dwelling near Gratuity Road. The area slopes towards Gratuity Road. Runoff from this area is undetained and untreated. The discharge point is designated as DP-2 on the existing HydroCAD analysis and accompanying drainage map.



#### 3.1.2 Existing Drainage Area Summary

The following table (Table 1) summarizes the existing drainage areas, including the pertinent information used for hydrologic analysis:

Drainage Area	Area (Acres)	Weighted Curve Number	* <b>Tc</b> (min)
EX-1A	30.7	30	104.9
EX-1B	20.8	35	43.9
EX-2	0.5	71	6.3

Table 1 – Existing Conditions Drainage Area Characteristics Summary

\*The minimum time of concentration used was 5.0 minutes.

#### 3.1.3 Peak Discharge Runoff Rates

The existing peak flow rates of stormwater runoff, tributary to the design point, were calculated for the 2-, 10-, 25-, and 100-year storm events. Results are presented in Table 3.

#### 3.2 **PROPOSED CONDITIONS**

As stated previously, the proposed project includes the construction of fourteen (14) duplex dwellings and two dead end roads. The proposed development will include clearing of vegetation, grading around the proposed buildings and driveways, and the construction of stormwater management BMP's. Stormwater controls are also proposed to attenuate peak rates and promote infiltration of runoff and are described in further detail in the following sections.

The development will include a Homeowner's Association (HOA) which will be responsible for the maintenance of all drainage systems. The annual maintenance cost is estimated to be \$6,000.

#### 3.2.1 **Proposed Drainage Areas**

**Proposed Drainage Areas PR-1A-B** consists of undisturbed woodland aeras and graded areas not captured by stormwater BMP's. Runoff flows overland undetained to the intermittent stream designated as DP-1. The discharge point is designated as DP-1 on the proposed HydroCAD analysis and accompanying drainage map.

**Proposed Drainage Areas PR-1C-f** consists of the majority of impervious areas and graded lawn areas. Runoff flows overland to several infiltration basins which have been designed to detain and infiltrate up to the 100-year storm. Infiltration overflow weirs direct flows towards DP-1. The discharge point is designated as DP-1 on the proposed HydroCAD analysis and accompanying drainage map.



**Proposed Drainage Areas PR-1G-H** consists of mostly undisturbed woodland and a portion of graded lawn areas. Runoff flows overland to several depressions that will be created by the proposed grading. Minor ponding is only anticipated in larger design storms.

**Proposed Drainage Area PR-2** consists of a portion of the proposed roadway and graded lawn. Runoff from impervious areas will flow overland undetained to Gratuity Road. The discharge point is designated as DP-2 on the proposed HydroCAD analysis and accompanying drainage map.

#### 3.2.2 Proposed Drainage Area Summary

The following table (Table 2) summarizes the proposed drainage areas, including the pertinent information used for hydrologic analysis:

Drainage Area	Area (Acres)	Weighted Curve	* <b>Tc</b> (min)
	(******)	Number	
PR-1A	21.7	32	104.9
PR-1B	17.2	33	43.9
PR-1C	0.7	80	5.0
PR-1D	2.4	81	5.0
PR-1E	1.0	79	5.0
PR-1F	1.8	82	5.0
PR-1G	3.0	40	24.8
PR-1H	2.5	41	24.8
PR-2	0.4	72	5.0

Table 2 – Proposed Conditions Drainage Area Characteristics Summary

\*The minimum time of concentration used was 5.0 minutes.



#### 3.2.3 Peak Discharge Runoff Rates

The peak flows were calculated for the 2-, 10-, 25-, and 100-year storm events under proposed conditions. The following table (Tables 3) represents a comparison between existing and proposed conditions of the peak rates of runoff from the project area to the discharge points.

Discharge Point		2-Year Storm (3.01")	10-Year Storm (4.44")	25-Year Storm (5.55")	100-Year Storm (7.80")
		Rate (cfs)	Rate (cfs)	Rate (cfs)	Rate (cfs)
DP-1	Existing	0.0	0.1	0.5	4.7
DP-1	Proposed	0.0	0.1	0.4	3.2
	Existing	0.3	0.9	1.3	2.3
DP-2	Proposed	0.3	0.7	1.1	1.9

Table 3 – Summary of Peak Flows

#### 3.2.4 Peak Discharge Volumes

The runoff volumes were calculated for the 2-, 10-, 25-, and 100-year storm events under proposed conditions. The following table (Tables 4) represents a comparison between existing and proposed conditions of the volume of runoff from the project area to the discharge points.

Discharge Point		2-Year Storm (3.01")	10-Year Storm (4.44")	25-Year Storm (5.55")	100-Year Storm (7.80")
Dischar		Volume (Cubic-Feet)	Volume (Cubic-Feet)	Volume (Cubic-Feet)	Volume (Cubic-Feet)
DP-1	Existing	0	975	10,272	75,466
DF-1	Proposed	0	69	7,285	59,196
	Existing	1,162	2,631	3,953	6,914
DP-2	Proposed	1,110	2,463	3,673	6,365



#### 3.3 METHODOLOGY AND DESIGN CRITERIA

#### 3.3.1 Hydrologic Model Description

The drainage analysis was performed using the Soil Conservation Service (SCS) TR-55 and TR-20 methodologies and the computer program HydroCAD 10.00 by HydroCAD Software Solutions, LLC.

#### 3.3.2 Design Storms

The analysis was performed on the 2-, 10-, 25-, and 100-year frequency rainfall events. The events were based on the 24-hour type-III duration storm.

#### 3.3.3 Time of Concentration

The 'time of concentration' ( $T_c$ ) for each watershed was determined by finding the time necessary for runoff to travel from the most hydraulically distant point in the watershed to the point of concentration. The travel path was drawn based on the topography and the time was calculated using the TR-55 Method and HydroCAD. A minimum  $T_c$  of 5.0 minutes was used.

#### 3.3.4 Curve Numbers

Curve numbers were developed for each of the different use categories and hydrologic soil group types within each sub-area. The curve numbers were based on the SCS TR-55 methodology and are included in the HydroCAD input and output found in the Attachments.

#### 3.3.5 Rainfall Depth

Rainfall depths were acquired from NRCC Extreme Precipitation Tables for Groton, Massachusetts. Rainfall events for the 2-, 10-, 25-, and 100-year storms were analyzed.

The following rainfall depths for Groton, Middlesex County, Massachusetts were used in the calculations:

Storm Event	Rainfall Depth
2-Year	3.01 inches
10-Year	4.44 inches
25-Year	5.55 inches
100-Year	7.80 inches



# 4.0 Standard 3: Recharge

Standard 3 requires that three computations or demonstrations be fulfilled in order to satisfy the stormwater recharge requirements, as follows:

- Impervious Area
- Required Recharge Volume
- Bottom Area Sizing for Infiltration Structures

These calculations are included in Appendix A.

#### 4.1 IMPERVIOUS AREA AND REQUIRED RECHARGE VOLUME

The first and second calculation for required recharge volume are based on the underlying soil types for the site and the amount of impervious area covering that soil type at the post-development site.

The proposed drainage design provides infiltration into the ground using the *Static* method for a specific volume based on the impervious areas over specific hydrologic soil groups. The required recharge calculations are calculated for the entire project area. The recharge calculations include the required recharge based on the post-development impervious coverage contributing to the infiltration basin and soil type.

The required recharge volume equals a depth of runoff corresponding to the soil type times the impervious areas covering that soil type at the post-development site. The Natural Resources Conservation Service (NRCS) Soil survey of Middlesex County, Massachusetts has mapped the soils on the project site.

#### 4.2 DRAWDOWN WITHIN 72 HOURS

The formula for calculating drawdown time for the infiltration basin is displayed below:

$$Time_{drawdown} = \frac{Rv}{(K)(Bottom \ Area)}$$



Where: *Rv* = Storage Volume *K* = Saturated Hydraulic Conductivity For "Static" and "Simple Dynamic" Methods, use *Rawls Rate (see Table 2.3.3). For "Dynamic Field" Method, use 50% of the in-situ saturated hydraulic conductivity. Bottom Area = Bottom Area of Recharge Structure* 

Calculations for drawdown time have been provided in Appendix A.



# 5.0 Standard 4: Water Quality

Standard 4 requires that all stormwater management systems be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS). The MA Stormwater Handbook states that this standard is met when:

- a. Suitable practices for source control and pollution prevention are identified in a longterm pollution prevention plan, and thereafter are implemented and maintained;
- b. Structural stormwater best management practices are sized to capture the required water quality volume as determined in accordance with the Massachusetts Stormwater Handbook; and
- c. Pretreatment is provided in accordance with the Massachusetts Stormwater Handbook.

#### 5.1 WATER QUALITY TREATMENT VOLUME

The project site is not located in an area of higher pollutant loading, therefore, a "Water Quality Depth" of 0.5" was used when computing the required water quality treatment volume. The calculation for determining the water quality treatment volume is as follows:

 $V_{WQ} = (D_{WQ}/12 \text{ inches/foot}) * (A_{IMP})$ 

$V_{WQ}$	= Required Water Quality Volume (in cubic feet)
$D_{WQ}$	= Water Quality Depth: 0.5"
A <sub>IMP</sub>	= Impervious Area (in square feet)

Calculations showing the required and provided water quality volume are included in Appendix A of this report.

### 5.2 TSS REMOVAL COMPUTATIONS

As required, a minimum of 80% TSS removal be achieved in the proposed condition. Appendix A contains calculations for TSS removal. Pre-treatment includes the use of two treatment trains: deep sump catch basins discharging to infiltration basins with sediment forebays; and proprietary separator structures discharging to infiltration basins with sediment forebays.



# 6.0 Standard 5: Land Uses with Higher Potential Pollutant Loads

For land uses with higher potential pollutant loads, source control and pollution prevention shall be implemented to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable.

The proposed project not considered a Land Use with Higher Potential Pollutant Loads (LUHPPL) and therefore Standard 5 does not apply.



# 7.0 Standard 6: Critical Areas

The project site is not considered a Critical Area and therefore Standard 6 is not applicable to this project.



# 8.0 Standard 7: Redevelopment

A redevelopment project is required to meet the following Stormwater Management Standards only to the maximum extent practicable: Standard 2, Standard 3, and the pretreatment and structural stormwater best management practice requirements of Standards 4, 5, and 6. Existing stormwater discharges shall comply with Standard 1 only to the maximum extent practicable. A redevelopment project shall also comply with all other requirements of the Stormwater Management Standards and improve existing conditions.

There is an increase of impervious area on the site and therefor this project is not considered a redevelopment project. Standard 7 is not applicable to the proposed project.



# 9.0 Standard 8: Construction Period Pollution Prevention and Erosion & Sedimentation Control

Construction period pollution prevention and erosion and sedimentation control measures will be implemented at the project site to control construction related impacts during construction and land disturbance activities. The general contractor for the project will be responsible for implementation of the construction period controls.

The project will disturb more than one acre of land during the construction process and will therefore require a NPDES Construction General Permit issued by the Environmental Protection Agency. As a result, a stormwater pollution prevention plan (SWPPP) will be required. The SWPPP document will satisfy the requirements of the Construction General Permit and the construction period erosion, sedimentation and pollution prevention plan requirements outlined in Standard 8 of the Massachusetts Stormwater Handbook.

Without proper erosion and sediment control measures, grading and filling may cause erosion and sedimentation, resulting in temporarily increased turbidity and suspended solid loads. Runoff from construction sites may also transport sediment to downstream watercourses, where sediment deposition and accumulation will occur as flow velocities decrease.

Erosion and sedimentation controls will be employed to prevent the erosion and transport of sediment into resource areas during the earthwork and construction phases of the project. Erosion and sedimentation control measures will be installed prior to site excavation or disturbance and will be maintained throughout the construction period.

Below is a description of some of the erosion and sediment control measures that will be employed at the project and that will be included in the SWPPP.

#### Silt Fence and Straw Wattles

Prior to any ground disturbance, a professional engineer or land surveyor will certify that a barrier of reinforced silt fence and straw wattles is in place at the down gradient limit of work in accordance with the site plan. The barrier will be placed to trap sediment transported by runoff before it reaches the drainage system or leaves the construction site. The silt fence is a semi-permeable barrier made of a synthetic porous fabric. When necessary, additional silt fence barriers will be installed immediately down gradient of erosion-prone areas, such as the base of steep exposed slopes, throughout the construction phase of the project. The barriers will be entrenched into the substrate to prevent underflow.

The erosion control barriers will be inspected weekly and after every storm event. Any sediment that collects behind the barriers will be removed and will be either reused at the site or disposed of at a suitable offsite location. Any damaged sections of silt fence or wattles will be repaired or replaced. The underside of the straw wattles will be kept in close contact with the earth and



reset as necessary. Straw wattles and silt fences will be maintained and cleaned until slopes have healthy stands of grass.

#### Dust Control

Fugitive dust from large areas of unstabilized soil can be a problem during construction. On dry and windy days when dust generation is a concern, a water truck will traverse the site and spray water as necessary to prevent dust from forming.

#### 9.1 MATERIAL MANAGEMENT PRACTICES

The following material management practices will be used to reduce the risk of spills or other accidental exposure of materials and substances to stormwater runoff. These include good housekeeping practices and guidelines for the handling of hazardous products. The following good housekeeping practices will be followed on-site during the construction period:

- An effort will be made to store only enough product required to do the job.
- All materials stored on-site will be stored in a neat, orderly manner in their appropriate containers, and (if possible) under a roof or other enclosure.
- Products will be kept in their original containers with the original manufacturer's label.
- Substances will not be mixed with one another unless recommended by the manufacturer.
- Whenever possible, all of a product will be used before disposing of the container.
- Manufacturer's recommendations for proper use and disposal will be followed.
- The site superintendent will inspect the storage area daily to ensure proper use and disposal of materials on-site.

#### Hazardous Products:

These practices will be used to reduce the risks associated with hazardous materials. Material Safety Data Sheets (MSDS) for each substance with hazardous properties that is used on the job site will be obtained and used for the proper management of potential wastes that may result from these products. An MSDS will be posted in the immediate area where such product is used and another copy of each MSDS will be maintained in the SWPPP file at the job site construction trailer office. Hazardous fuels or other potential contaminants shall not be stored on site. Each employee who must handle a substance with hazardous properties will be instructed on the use of MSDS sheets and the specific information in the applicable MSDS for the product they are using, particularly regarding spill control techniques.

- Products will be kept in original containers unless they are not re-sealable
- Original labels and material safety data will be retained; they contain important product information



• If surplus product must be disposed of, manufacturer's or local and State recommended methods for proper disposal will be followed

#### Hazardous Waste

All hazardous waste material will be disposed of by the Contractor in the manner specified by local, state, and/or federal regulations and by the manufacturer of such products. Site personnel will be instructed in these practices by the job site superintendent, who will also be responsible for seeing that these practices are followed.

#### Solid and Construction Wastes

All waste materials will be collected and stored in accordance with state and federal law in an appropriately covered container and/or securely lidded metal dumpster.

All trash and construction debris from the site will be transported off site. No construction waste materials will be buried on site. All personnel will be instructed regarding the correct procedures for waste disposal.

#### Sanitary Wastes

All sanitary waste will be collected from the portable units as required to maintain proper operation and sanitary conditions of these units. All maintenance work on portable sanitation units shall be performed by a licensed portable facility provider in complete compliance with local and state regulations.

All sanitary waste units will be located in an area where the likelihood of the unit contributing to storm water discharges is negligible. If required, additional BMPs must be implemented, such as sandbags around the base, to prevent wastes from contributing to storm water discharges.

#### 9.2 **PRODUCT SPECIFIC PRACTICES**

The following product-specific practices will be followed on-site. Recommendations are provided for petroleum products, fertilizers, solvents, paints, and other hazardous substances.

#### Petroleum Products

All on-site vehicles will be monitored for leaks and will receive regular preventive maintenance to reduce the chance of leakage. Petroleum products will be stored in tightly sealed containers that are clearly labeled. Any asphalt substances used on-site will be applied according to manufacturer's recommendations.

#### Solvents, Paints, and other Hazardous Substances

All containers will be tightly sealed and stored when not required for use. Excess materials will not be discharged to the storm sewer system, but will be properly disposed of according to



manufacturer's instructions or state and local regulations. No storage will occur within 100 feet of a resource area.

#### 9.3 SPILL CONTROL/NOTIFICATION PRACTICES

In addition to the good housekeeping and material management practices discussed above, the following practices will be followed for spill control, notification and cleanup.

- Manufacturer's recommended methods for spill cleanup will be clearly posted and site personnel will be informed of the procedures and the location of the information and cleanup supplies.
- Materials and equipment necessary for spill cleanup will be kept in the material storage area on-site. Equipment and materials will include, but will not be limited to: Shovels, wheel barrows, brooms, dust pans, mops, rags, gloves, goggles, kitty litter or Speedi-Dry, sand, sawdust, and plastic and metal trash containers specifically designated for this purpose.
- All spills will be cleaned up immediately after discovery.
- The spill area will be kept well-ventilated and personnel will wear protective clothing to prevent injury from contact with a hazardous substance.
- Spills of toxic or hazardous material in excess of reportable quantities, as established in the Massachusetts Contingency Plan (MCP), will be reported to the Massachusetts Department of Environmental Protection Division of Hazardous Waste [(617) 292-5851 or (978) 661-7679].
- The construction superintendent responsible for the daily operations will be the spill prevention and cleanup coordinator. He will designate at least three other site personnel to receive spill prevention and cleanup training. These individuals will each become responsible for a particular phase of prevention and cleanup. The names of the responsible spill personnel will be posted in the material storage area and in the on-site office trailer.



# 10.0 Standard 9: Operation and Maintenance Plan

The goal of the operation and maintenance plan is to protect resources in the region that may be affected by the activities at the site. Water quality treatment measures and the implementation of Best Management Practices (BMP's) for structural controls will result in the treatment of site stormwater.

The stormwater management system will be a portion of the area leased by the applicant. They will be responsible for operation and maintenance. The estimated operation and maintenance budget is expected to be about \$2,000.

#### 10.1 NON-STRUCTURAL POLLUTANT CONTROLS

The proposed stormwater management system is designed to protect the runoff water quality through the removal of sediment and pollutants. Non-structural pollutant controls used to separate and capture stormwater pollutants are described below.

#### Deicing Chemicals

The use of any deicing chemicals will be used sparingly and will follow the manufacturer's recommendations for application.

#### <u>Fertilizer</u>

Slow-release organic fertilizers will be used in the landscaped areas to limit the amount of nutrients that could enter downstream resource areas. Fertilizer use will be reduced once proposed landscaping is established.

#### Street Sweeping

Street dirt accumulates on roads and parking lots and runs off in response to precipitation. Street sweeping will occur on the project site as necessary with a minimum frequency of twice per year to control sediment, dust, and sand. Sweeping will be scheduled primarily in the spring and fall.

### 10.2 STRUCTURAL POLLUTANT CONTROLS

The proposed stormwater management system is designed to protect runoff water quality through the removal of sediment and pollutants. Structural pollutant controls used to separate and capture stormwater pollutants are described below.



#### VEGETATED AREAS MAINTENANCE

Although not a structural component of the drainage system, the maintenance of vegetated areas may affect the functioning of stormwater management practices. This includes the health/density of vegetative cover and activities such as the application and disposal of lawn and garden care products, disposal of leaves and yard trimmings.

#### DEEP SUMP CATCH BASIN

*Purpose:* The stormwater management system includes the use of deep sump catch basins to enhance total suspended solids removal. The proper function of these items is crucial to providing adequate groundwater recharge and flood control.

*Minimum required maintenance:* The maintenance of the individual catch basins affects how well the stormwater management system performs and helps with longevity. Inspect or clean deep sump basins at least four times per year and at the end of the foliage and snow removal seasons. Sediments must also be removed four times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin.

#### CONTECH CDS HYDRODYNAMIC SEPARATOR

*Purpose:* The stormwater management system includes Contech CDS systems which are hydrodynamic separators used to remove pollutants from stormwater runoff, including suspended solids and free oils. It is important that continued maintenance of this infrastructure is performed to ensure that it will continue to function as designed.

*Minimum required maintenance:* Inspection should be performed at least twice per year, though the frequency of maintenance may need to be increased or reduced based on local conditions. Maintenance shall be performed when the level of sediment has reached 75% of capacity in the isolated sump or when an appreciable level of hydrocarbons and trash has accumulated.

#### RAIN GUARDIAN TURRET PRETREATMENT STRUCTURE

*Purpose:* The stormwater management system includes the use of proprietary pretreatment structures to enhance total suspended solids removal. It is important that continued maintenance of this infrastructure is performed to ensure that it will continue to function as designed.

*Minimum required maintenance:* Regular maintenance is essential. Inspect or clean drain separator structures similar to catch basins, at least four times per year. Sediments must be removed whenever the depth of deposits is greater than or equal to one half the depth from bottom of sump to outlet elevation. Sediment to be removed from collection chamber with shovels or with hydro-vac, and drop in filters to be cleaned with broom or hose.



#### DRAIN PIPES AND OVERFLOW WEIRS

*Purpose:* The stormwater management system includes the use of drainpipes and an overflow weir to convey stormwater. It is important that continued maintenance of this infrastructure is performed to ensure that it will continue to function as designed.

*Minimum required maintenance:* Regular maintenance is essential. Inspect or clean drain pipes/overflow pipe similar to catch basins, at least four times per year. Sediments must be removed whenever the depth of deposits is greater than or equal to one half the depth of the pipe. Clean trash racks, inlet and outlet structures similarly.

#### SEDIMENT FOREBAY

*Purpose:* The stormwater management system includes sediment forebays which are excavated pits and/or bermed areas. They are designed to slow incoming stormwater runoff allowing for the settlement of suspended solids. The maintenance of the system is critical as they do not function properly with sediment buildup.

*Minimum required maintenance:* Visual inspection of the sediment forebay shall occur monthly and cleaned out at least four (4) times per year. Mow grass on a regular/seasonal basis removing clippings and accumulated sediment from the surface.

Check for signs of rilling and gullying and repair as necessary. Replace any damaged vegetation immediately.

#### **INFILTRATION BASIN**

*Purpose:* The stormwater management system includes an infiltration basin to provide water quality treatment and recharge, as well as attenuate peak flows. The proposed infiltration basin has been designed to allow adequate separation to groundwater. The maintenance of the system is critical as they are prone to clogging and failure.

*Minimum required maintenance:* Visual inspection of the infiltration trench will occur twice per year and after every major storm during the first 3 months of operation. Remove any debris that might clog the system. If water is observed and it is at least 72 hours after a rain event, the system will be cleaned to remove any built-up sediment.

A major storm event is defined as a storm that is equal to or greater than the 2-year, 24-hour storm (generally 3.1 inches in a 24-hour period).

Important items to check during the inspection include:

- Signs of differential settlement,
- Cracking,
- Erosion,
- Leakage in the embankments
- Tree growth on the embankments
- Condition of riprap,
- Sediment accumulation and the health of the turf.

At least twice a year, mow the buffer area, side slopes, and basin bottom. Remove grass clippings and accumulated organic matter to prevent an impervious organic mat from forming.



Remove trash and debris at the same time. Use deep tilling to break up clogged surfaces and revegetate immediately.

Remove sediment from the basin as necessary when the floor of the basin is thoroughly dry. Use light equipment to remove the top layer to not compact the underlying soil. Deeply till the remaining soil and revegetate as soon as possible.

#### **References**

Massachusetts Department of Environmental Protection (MassDEP) Stormwater Handbook, Volume 2, Chapter 2, Structural BMP Specifications for Massachusetts Stormwater Handbook, revised and updated February 2008.



# 11.0 Standard 10: Prohibition of Illicit Discharges

Standard 10 of the Massachusetts Stormwater Handbook prohibits illicit discharges to stormwater management systems. As stated in the handbook, "The stormwater management system is the system for conveying, treating, and infiltrating stormwater on-site, including stormwater best management practices and any pipes intended to transport stormwater to the groundwater, a surface water, or municipal separate storm sewer system. Illicit discharges to the stormwater management system are discharges that are not entirely comprised of stormwater."

Proponents of projects within Wetlands jurisdiction must demonstrate compliance with this requirement by submitting to the issuing authority an Illicit Discharge Compliance Statement verifying that no illicit discharges exist on the site and by including in the pollution prevention plan measures to prevent illicit discharges to the stormwater management system. An Illicit Discharge Compliance Statement for the project follows:

#### **Illicit Discharge Compliance Statement**

Per the requirements of Standard 10 of the Massachusetts Stormwater Management Standards it shall be stated that <u>No Illicit Discharges exist</u> on the project site located at 63 Gratuity Road in Groton, Massachusetts.

hull he

Name

15/22 Date

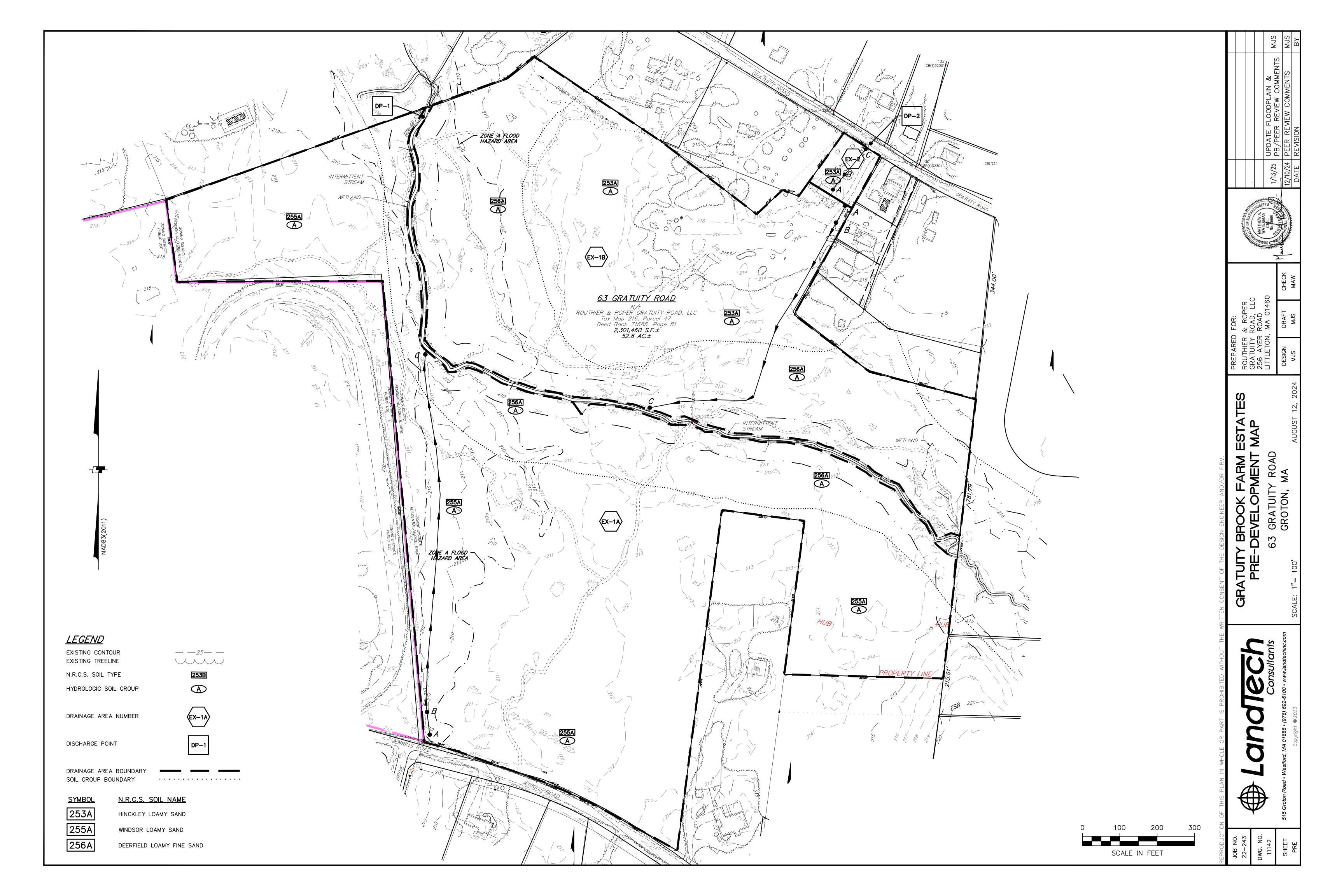


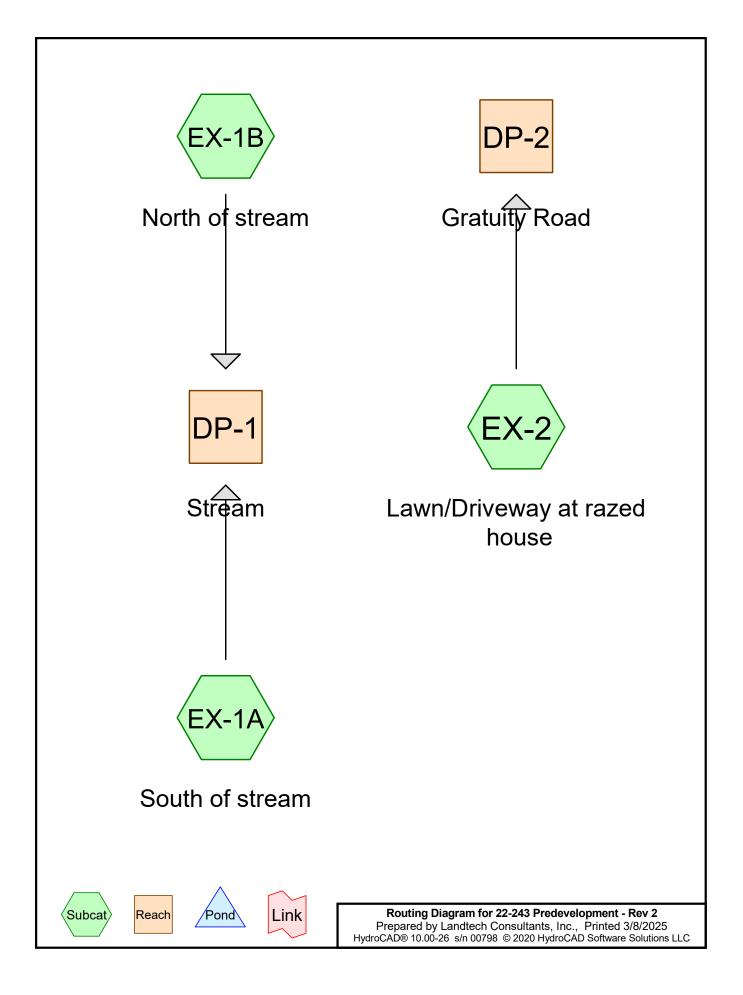
# **APPENDIX A. CALCULATIONS**

Existing Conditions (HydroCAD) Proposed Conditions (HydroCAD) Recharge Volume Calculations (Part I and II) Water Quality Treatment Volume Calculations TSS Calculations Sediment Forebay Sizing Calculations Pipe Calculations Groundwater Mounding Analysis



**Existing Conditions (HydroCAD)** 





### Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
129,569	68	<50% Grass cover, Poor, HSG A (EX-1B, EX-2)
9,402	98	Paved parking, HSG A (EX-1B, EX-2)
2,125,132	30	Woods, Good, HSG A (EX-1A, EX-1B)
2,264,103	32	TOTAL AREA

## Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
2,264,103	HSG A	EX-1A, EX-1B, EX-2
0	HSG B	
0	HSG C	
0	HSG D	
0	Other	
2,264,103		TOTAL AREA

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HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Sub
(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover	Nur
129,569	0	0	0	0	129,569	<50% Grass	
						cover, Poor	
9,402	0	0	0	0	9,402	Paved parking	
2,125,132	0	0	0	0	2,125,132	Woods, Good	
2,264,103	0	0	0	0	2,264,103	TOTAL AREA	
	(sq-ft) 129,569 9,402 2,125,132	(sq-ft)         (sq-ft)           129,569         0           9,402         0           2,125,132         0	(sq-ft)         (sq-ft)         (sq-ft)           129,569         0         0           9,402         0         0           2,125,132         0         0	(sq-ft)(sq-ft)(sq-ft)129,569009,402002,125,13200	(sq-ft)(sq-ft)(sq-ft)(sq-ft)129,56900009,40200002,125,1320000	(sq-ft)(sq-ft)(sq-ft)(sq-ft)(sq-ft)129,5690000129,5699,40200009,4022,125,13200002,125,132	(sq-ft)(sq-ft)(sq-ft)(sq-ft)(sq-ft)Cover129,5690000129,569<50% Grass cover, Poor9,40200009,402Paved parking2,125,13200002,125,132Woods, Good

#### Ground Covers (all nodes)

22-243 Predevelopment - Rev 2	Type III 24-hr 2 Year Rainfall=3.01"					
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HydroCAD® 10.00-26 s/n 00798 © 2020 Hyd	droCAD Software Solutions LLC Page 5					
Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method						
Subcatchment EX-1A: South of stream Flow Length=1,0	Runoff Area=1,335,882 sf 0.00% Impervious Runoff Depth=0.00" 035' Slope=0.0020 '/' Tc=104.9 min CN=30 Runoff=0.00 cfs 0 cf					
Subcatchment EX-1B: North of stream	Runoff Area=908,040 sf 0.78% Impervious Runoff Depth=0.00" Flow Length=860' Tc=43.9 min CN=35 Runoff=0.00 cfs 0 cf					
Subcatchment EX-2: Lawn/Driveway at	Runoff Area=20,181 sf 11.48% Impervious Runoff Depth>0.69" Flow Length=160' Tc=6.3 min CN=71 Runoff=0.36 cfs 1,162 cf					
Reach DP-1: Stream	Inflow=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf					
Reach DP-2: Gratuity Road	Inflow=0.36 cfs 1,162 cf Outflow=0.36 cfs 1,162 cf					

Total Runoff Area = 2,264,103 sfRunoff Volume = 1,162 cfAverage Runoff Depth = 0.01"99.58% Pervious = 2,254,701 sf0.42% Impervious = 9,402 sf

#### Summary for Subcatchment EX-1A: South of stream

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

A	rea (sf)	CN E	Description		
1,3	35,882	30 V	Voods, Go	od, HSG A	
1,3	1,335,882 100.00% Pervious Area		ervious Are	a	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.5	50	0.0020	0.03		Sheet Flow, A-B
73.4	985	0.0020	0.22		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow, B-C</b> Woodland Kv= 5.0 fps
104.9	1,035	Total			

#### Summary for Subcatchment EX-1B: North of stream

Runoff = 0.00 cfs @ 5.00 hrs, Volume=

0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

_	A	rea (sf)	CN [	Description				
	7	89,250	30 \	30 Woods, Good, HSG A				
	1	11,705	68 <	<50% Gras	s cover, Po	or, HSG A		
_		7,085	98 F	Paved park	ing, HSG A			
	9	08,040	35 \	Veighted A	verage			
	9	00,955	ç	9.22% Per	vious Area			
		7,085 0.78% Impervious Area				а		
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	5.7	50	0.0200	0.15		Sheet Flow, A-B		
_	38.2	810	0.0050	0.35		Grass: Short n= 0.150 P2= 3.10" Shallow Concentrated Flow, B-C Woodland Kv= 5.0 fps		
	120	000	Tatal					

43.9 860 Total

#### Summary for Subcatchment EX-2: Lawn/Driveway at razed house

Runoff = 0.36 cfs @ 12.11 hrs, Volume= 1,162 cf, Depth> 0.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

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Type III 24-hr 2 Year Rainfall=3.01" Printed 3/8/2025 LLC Page 7

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_	A	rea (sf)	CN Description						
		17,864	68	68 <50% Grass cover, Poor, HSG A					
_		2,317	98	Paved park	ing, HSG A				
		20,181 71 Weighted Average							
	17,864 88.52% Pervious Area								
	2,317 11.48% Impervious Area					ea			
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)		(cfs)	Description			
-	3.7	50	0.0600	0.23		Sheet Flow, A-B			
						Grass: Short n= 0.150 P2= 3.10"			
	2.6	110	0.0100	0.70		Shallow Concentrated Flow, B-C			
_						Short Grass Pasture Kv= 7.0 fps			
	6.0	100	Tatal						

6.3 160 Total

#### Summary for Reach DP-1: Stream

Inflow Area =	=	2,243,922 sf,	0.32% Impervious,	Inflow Depth = 0.00"	for 2 Year event
Inflow =		0.00 cfs @	5.00 hrs, Volume=	0 cf	
Outflow =		0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

#### Summary for Reach DP-2: Gratuity Road

Inflow Area	a =	20,181 sf,	11.48% Impervious,	Inflow Depth > 0.69"	for 2 Year event
Inflow	=	0.36 cfs @	12.11 hrs, Volume=	1,162 cf	
Outflow	=	0.36 cfs @	12.11 hrs, Volume=	1,162 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

<b>22-243 Predevelopment - Rev 2</b> Prepared by Landtech Consultants, Inc HydroCAD® 10.00-26 s/n 00798 © 2020 Hydrocechecker	
Runoff by SCS T	00-20.00 hrs, dt=0.05 hrs, 301 points R-20 method, UH=SCS, Weighted-CN Frans method - Pond routing by Stor-Ind method
Subcatchment EX-1A: South of stream Flow Length=1,0	Runoff Area=1,335,882 sf 0.00% Impervious Runoff Depth=0.00" )35' Slope=0.0020 '/' Tc=104.9 min CN=30 Runoff=0.00 cfs 0 cf
Subcatchment EX-1B: North of stream	Runoff Area=908,040 sf 0.78% Impervious Runoff Depth>0.01" Flow Length=860' Tc=43.9 min CN=35 Runoff=0.07 cfs 975 cf
Subcatchment EX-2: Lawn/Driveway at	Runoff Area=20,181 sf 11.48% Impervious Runoff Depth>1.56" Flow Length=160' Tc=6.3 min CN=71 Runoff=0.88 cfs 2,631 cf
Reach DP-1: Stream	Inflow=0.07 cfs 975 cf Outflow=0.07 cfs 975 cf
Reach DP-2: Gratuity Road	Inflow=0.88 cfs 2,631 cf Outflow=0.88 cfs 2,631 cf

Total Runoff Area = 2,264,103 sfRunoff Volume = 3,606 cfAverage Runoff Depth = 0.02"99.58% Pervious = 2,254,701 sf0.42% Impervious = 9,402 sf

# Summary for Subcatchment EX-1A: South of stream

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

	A	rea (sf)	CN E	Description		
	1,3	35,882	30 V	Voods, Go	od, HSG A	
-	1,3	35,882	1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	31.5	50	0.0020	0.03		Sheet Flow, A-B
	73.4	985	0.0020	0.22		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow, B-C</b> Woodland Kv= 5.0 fps
	104.0	1 0 2 5	Total			

104.9 1,035 Total

### Summary for Subcatchment EX-1B: North of stream

Runoff = 0.07 cfs @ 19.95 hrs, Volume=

975 cf, Depth> 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

_	А	rea (sf)	CN [	Description		
	7	89,250	30 \	Voods, Go	od, HSG A	
	1	11,705	68 <	<50% Gras	s cover, Po	bor, HSG A
_		7,085	98 F	Paved park	ing, HSG A	
	9	08,040	35 \	Veighted A	verage	
	9	00,955	ç	9.22% Per	vious Area	
		7,085	(	).78% Impe	ervious Area	a
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	5.7	50	0.0200	0.15		Sheet Flow, A-B
_	38.2	810	0.0050	0.35		Grass: Short n= 0.150 P2= 3.10" Shallow Concentrated Flow, B-C Woodland Kv= 5.0 fps
	120	000	Tatal			

43.9 860 Total

#### Summary for Subcatchment EX-2: Lawn/Driveway at razed house

Runoff = 0.88 cfs @ 12.10 hrs, Volume= 2,631 cf, Depth> 1.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

# 22-243 Predevelopment - Rev 2

Type III 24-hr 10 Year Rainfall=4.44"Printed 3/8/2025s LLCPage 10

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_	A	rea (sf)	CN E	Description						
		17,864	68 <	<50% Grass cover, Poor, HSG A						
_		2,317	98 F	Paved park	ing, HSG A	\				
		20,181	71 V							
		17,864	8	8.52% Per	vious Area					
		2,317	1	1.48% Imp	pervious Are	ea				
	Тс	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	3.7	50	0.0600	0.23		Sheet Flow, A-B				
						Grass: Short n= 0.150 P2= 3.10"				
	2.6	110	0.0100	0.70		Shallow Concentrated Flow, B-C				
_						Short Grass Pasture Kv= 7.0 fps				
	6.2	160	Tatal							

6.3 160 Total

# Summary for Reach DP-1: Stream

Inflow Area =	2,243,922 sf,	0.32% Impervious,	Inflow Depth >	0.01"	for 10 Year event
Inflow =	0.07 cfs @	19.95 hrs, Volume=	975 cf		
Outflow =	0.07 cfs @	19.95 hrs, Volume=	975 cf	, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

# Summary for Reach DP-2: Gratuity Road

Inflow Area	a =	20,181 sf, 11.48% Impervio	ous, Inflow Depth > 1.56"	for 10 Year event
Inflow	=	0.88 cfs @ 12.10 hrs, Volum	ne= 2,631 cf	
Outflow	=	0.88 cfs @ 12.10 hrs, Volum	ne= 2,631 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

<b>22-243 Predevelopment - Rev 2</b> Prepared by Landtech Consultants, Ind HydroCAD® 10.00-26 s/n 00798 © 2020 Hy						
Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method						
Subcatchment EX-1A: South of stream Flow Length=1,035'	Runoff Area=1,335,882 sf 0.00% Impervious Runoff Depth>0.01" Slope=0.0020 '/' Tc=104.9 min CN=30 Runoff=0.11 cfs 1,261 cf					
Subcatchment EX-1B: North of stream	Runoff Area=908,040 sf 0.78% Impervious Runoff Depth>0.12" Flow Length=860' Tc=43.9 min CN=35 Runoff=0.46 cfs 9,011 cf					
Subcatchment EX-2: Lawn/Driveway at	Runoff Area=20,181 sf 11.48% Impervious Runoff Depth>2.35" Flow Length=160' Tc=6.3 min CN=71 Runoff=1.34 cfs 3,953 cf					
Reach DP-1: Stream	Inflow=0.46 cfs 10,272 cf Outflow=0.46 cfs 10,272 cf					
Reach DP-2: Gratuity Road	Inflow=1.34 cfs 3,953 cf Outflow=1.34 cfs 3,953 cf					
	of Dunoff Valume = 44,225 of Average Dunoff Double = 0.00"					

Total Runoff Area = 2,264,103 sf Runoff Volume = 14,225 cfAverage Runoff Depth = 0.08"99.58% Pervious = 2,254,701 sf0.42% Impervious = 9,402 sf

# Summary for Subcatchment EX-1A: South of stream

Runoff = 0.11 cfs @ 20.00 hrs, Volume= 1,261 cf, Depth> 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

A	rea (sf)	CN E	Description		
1,3	335,882	30 V	Voods, Go	od, HSG A	
1,3	335,882	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
31.5	50	0.0020	0.03		Sheet Flow, A-B
73.4	985	0.0020	0.22		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow, B-C</b> Woodland Kv= 5.0 fps
104.9	1,035	Total			

# Summary for Subcatchment EX-1B: North of stream

Runoff = 0.46 cfs @ 14.95 hrs, Volume= 9,011 cf, Depth> 0.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

	A	rea (sf)	CN I	Description					
-	7	89,250	30 \	Woods, Good, HSG A					
	1	11,705	68 •	<50% Ġras	s cover, Po	oor, HSG A			
_		7,085	98 I	Paved park	ing, HSG A				
	9	08,040	35 \	Neighted A	verage				
	9	00,955	ę	99.22% Per	vious Area				
		7,085	(	).78% Impe	ervious Area	а			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	5.7	50	0.0200	0.15		Sheet Flow, A-B			
_	38.2	810	0.0050	0.35		Grass: Short n= 0.150 P2= 3.10" Shallow Concentrated Flow, B-C Woodland Kv= 5.0 fps			
_	120	060	Total						

43.9 860 Total

#### Summary for Subcatchment EX-2: Lawn/Driveway at razed house

Runoff = 1.34 cfs @ 12.10 hrs, Volume= 3,953 cf, Depth> 2.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

# 22-243 Predevelopment - Rev 2

Type III 24-hr 25 Year Rainfall=5.55" Printed 3/8/2025 s LLC Page 13

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_	A	rea (sf)	CN E	Description						
		17,864	68 <	<50% Grass cover, Poor, HSG A						
_		2,317	98 F	aved park	ing, HSG A	\				
		20,181	71 Weighted Average							
		17,864	8	8.52% Per	vious Area					
		2,317	1	1.48% Imp	pervious Are	ea				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	3.7	50	0.0600	0.23		Sheet Flow, A-B				
						Grass: Short n= 0.150 P2= 3.10"				
	2.6	110	0.0100	0.70		Shallow Concentrated Flow, B-C				
_						Short Grass Pasture Kv= 7.0 fps				
	0.0	400	Tatal							

6.3 160 Total

# Summary for Reach DP-1: Stream

Inflow Area =	2,243,922 sf,	0.32% Impervious,	Inflow Depth > 0.05"	for 25 Year event
Inflow =	0.46 cfs @ 1	14.97 hrs, Volume=	10,272 cf	
Outflow =	0.46 cfs @ 1	14.97 hrs, Volume=	10,272 cf, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

# Summary for Reach DP-2: Gratuity Road

Inflow Are	a =	20,181 sf, 11.48% Impervious, Inflow Depth > 2.35" for 25 Yea	r event
Inflow	=	1.34 cfs @ 12.10 hrs, Volume= 3,953 cf	
Outflow	=	1.34 cfs @ 12.10 hrs, Volume= 3,953 cf, Atten= 0%, Lag=	: 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

<b>22-243 Predevelopment - Rev 2</b> Prepared by Landtech Consultants, Ir	Type III 24-hr 100 Year Rainfall=7.80" Printed 3/8/2025
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Runoff by SCS	5.00-20.00 hrs, dt=0.05 hrs, 301 points TR-20 method, UH=SCS, Weighted-CN +Trans method - Pond routing by Stor-Ind method
Subcatchment EX-1A: South of stream Flow Length=1,035'	Runoff Area=1,335,882 sf 0.00% Impervious Runoff Depth>0.26" Slope=0.0020 '/' Tc=104.9 min CN=30 Runoff=1.55 cfs 29,368 cf
Subcatchment EX-1B: North of stream	Runoff Area=908,040 sf 0.78% Impervious Runoff Depth>0.61" Flow Length=860' Tc=43.9 min CN=35 Runoff=4.51 cfs 46,078 cf
Subcatchment EX-2: Lawn/Driveway at	Runoff Area=20,181 sf 11.48% Impervious Runoff Depth>4.11" Flow Length=160' Tc=6.3 min CN=71 Runoff=2.33 cfs 6,914 cf
Reach DP-1: Stream	Inflow=4.67 cfs 75,446 cf Outflow=4.67 cfs 75,446 cf
Reach DP-2: Gratuity Road	Inflow=2.33 cfs 6,914 cf Outflow=2.33 cfs 6,914 cf

Total Runoff Area = 2,264,103 sfRunoff Volume = 82,360 cfAverage Runoff Depth = 0.44"99.58% Pervious = 2,254,701 sf0.42% Impervious = 9,402 sf

# Summary for Subcatchment EX-1A: South of stream

Runoff = 1.55 cfs @ 14.85 hrs, Volume= 29,368 cf, Depth> 0.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

	A	rea (sf)	CN E	Description		
	1,3	35,882	30 V	Voods, Go	od, HSG A	
-	1,3	35,882	1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	31.5	50	0.0020	0.03		Sheet Flow, A-B
	73.4	985	0.0020	0.22		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow, B-C</b> Woodland Kv= 5.0 fps
	101 0	1 0 2 5	Tatal			

104.9 1,035 Total

#### Summary for Subcatchment EX-1B: North of stream

Runoff = 4.51 cfs @ 12.85 hrs, Volume= 46,078 cf, Depth> 0.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

	A	rea (sf)	CN I	Description		
	7	89,250	30 \	Noods, Go	od, HSG A	
	1	11,705	68 •	<50% Gras	s cover, Po	oor, HSG A
_		7,085	98 I	Paved park	ing, HSG A	
	9	08,040	35 \	Neighted A	verage	
	9	00,955	ę	99.22% Per	vious Area	
		7,085	(	).78% Impe	ervious Area	a
	Та	l a sa aith	Clana	Valacity	Conseitu	Description
	Tc	Length	Slope	,	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.7	50	0.0200	0.15		Sheet Flow, A-B
						Grass: Short n= 0.150 P2= 3.10"
	38.2	810	0.0050	0.35		Shallow Concentrated Flow, B-C
_						Woodland Kv= 5.0 fps
	42.0	000	Tatal			

43.9 860 Total

#### Summary for Subcatchment EX-2: Lawn/Driveway at razed house

Runoff = 2.33 cfs @ 12.10 hrs, Volume= 6,914 cf, Depth> 4.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

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 Type III 24-hr
 100 Year Rainfall=7.80"

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_	A	rea (sf)	CN	Description	า	
		17,864	68	<50% Gras	s cover, Po	bor, HSG A
_		2,317	98	Paved parl	king, HSG A	
_		20,181	71	Weighted /	Average	
		17,864		88.52% Pe	rvious Area	l de la constante de
		2,317		11.48% Im	pervious Ar	ea
	Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	Description
	3.7	50	0.060	0 0.23		Sheet Flow, A-B
	2.6	110	0.010	0 0.70		Grass: Short n= 0.150 P2= 3.10" <b>Shallow Concentrated Flow, B-C</b> Short Grass Pasture Kv= 7.0 fps
_	0.0	400	Tatal			

6.3 160 Total

# Summary for Reach DP-1: Stream

Inflow Area =	2,243,922 sf	, 0.32% Impervious,	Inflow Depth > 0.40"	for 100 Year event
Inflow =	4.67 cfs @	12.87 hrs, Volume=	75,446 cf	
Outflow =	4.67 cfs @	12.87 hrs, Volume=	75,446 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

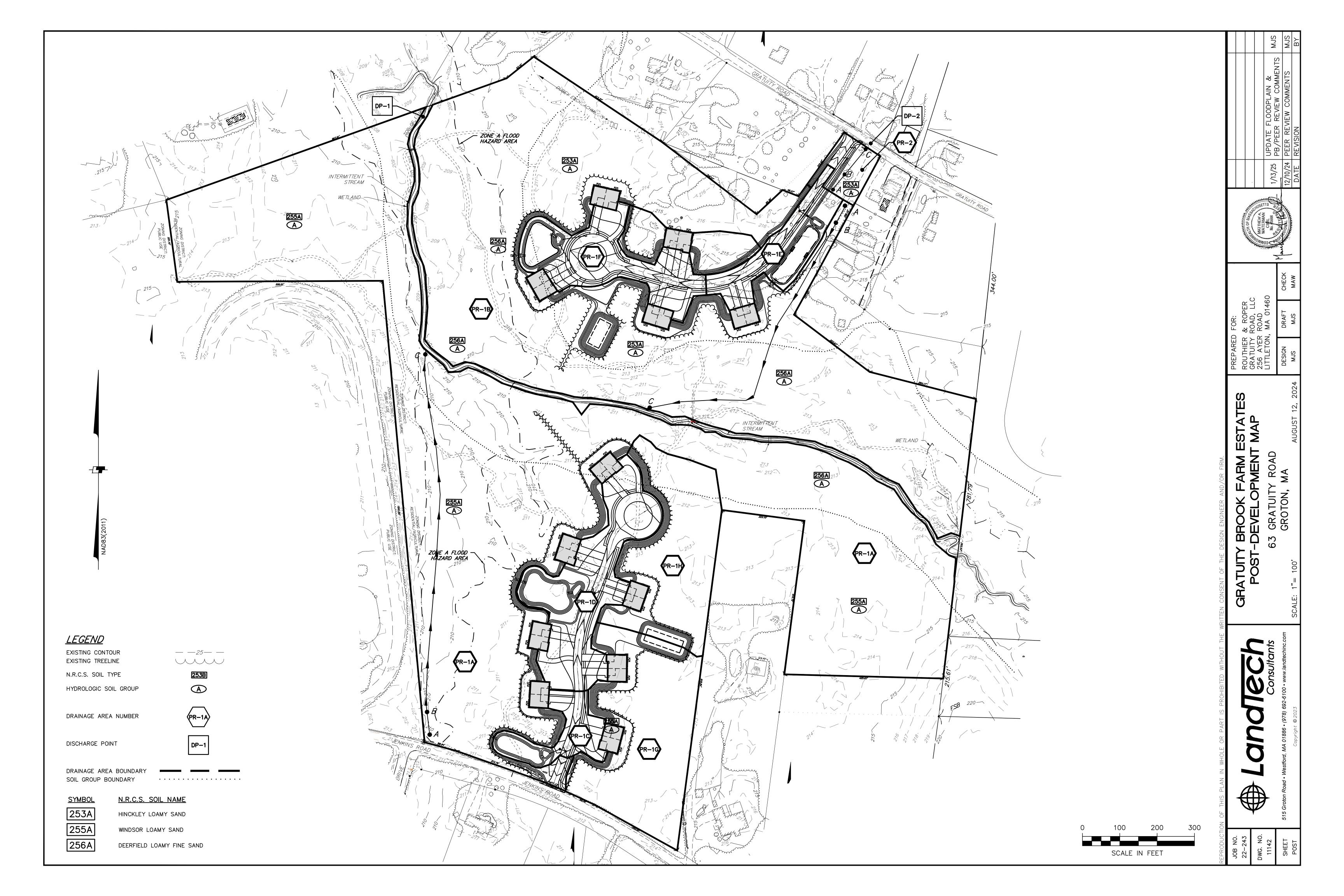
# Summary for Reach DP-2: Gratuity Road

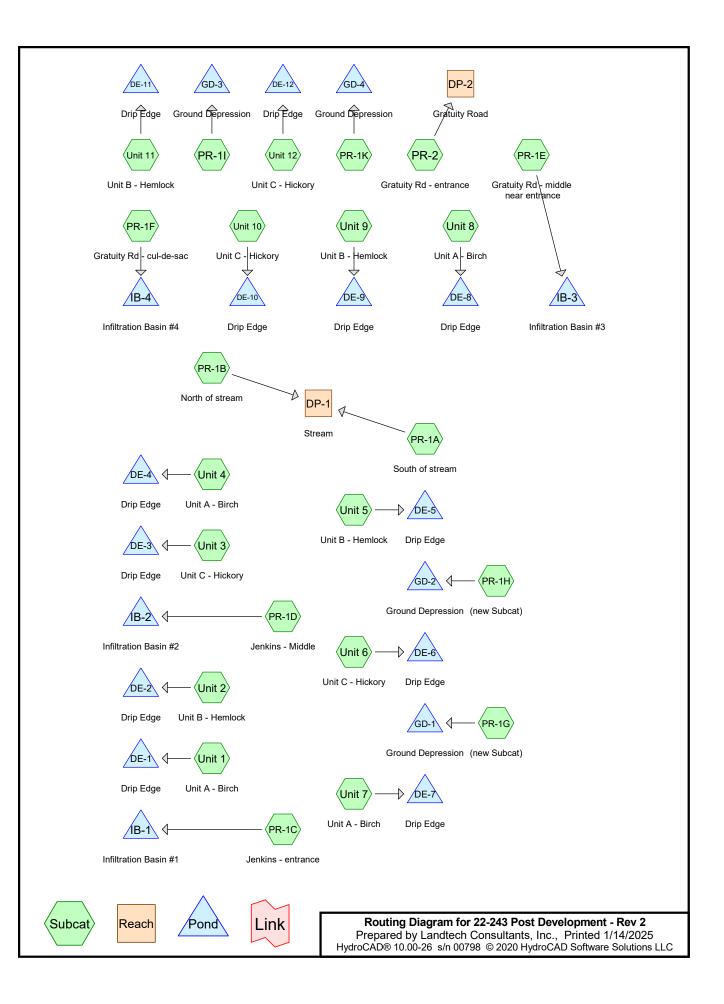
Inflow Area	ı =	20,181 sf	, 11.48% Impervious	Inflow Depth > 4.1	1" for 100 Year event
Inflow	=	2.33 cfs @	12.10 hrs, Volume=	6,914 cf	
Outflow	=	2.33 cfs @	12.10 hrs, Volume=	6,914 cf, A	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



**Proposed Conditions (HydroCAD)** 





# Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
359,253	68	<50% Grass cover, Poor, HSG A (PR-1A, PR-1B, PR-1C, PR-1D, PR-1E,
		PR-1F, PR-1G, PR-1H, PR-1I, PR-1K, PR-2)
44,590	30	Meadow, non-grazed, HSG A (PR-1B, PR-1K)
1,947	98	Paved parking, HSG A (PR-1A)
89,974	98	Paved roads w/curbs & sewers, HSG A (PR-1C, PR-1D, PR-1E, PR-1F, PR-2)
44,000	98	Roofs, HSG A (PR-1C, PR-1D, PR-1E, PR-1F, Unit 1, Unit 10, Unit 11, Unit 12,
		Unit 2, Unit 3, Unit 4, Unit 5, Unit 6, Unit 7, Unit 8, Unit 9)
1,724,339	30	Woods, Good, HSG A (PR-1A, PR-1B, PR-1G, PR-1H, PR-1I)
2,264,103	40	TOTAL AREA

# Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
2,264,103	HSG A	PR-1A, PR-1B, PR-1C, PR-1D, PR-1E, PR-1F, PR-1G, PR-1H, PR-1I, PR-1K, PR-2, Unit 1, Unit 10, Unit 11, Unit 12, Unit 2, Unit 3, Unit 4, Unit 5, Unit 6, Unit 7, Unit 8, Unit 9
0	HSG B	
0	пось	
0	HSG C	
0	HSG D	
0	Other	
2,264,103		TOTAL AREA

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HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover
359,253	0	0	0	0	359,253	<50% Grass cover, Poor
44,590	0	0	0	0	44,590	Meadow, non-grazed
1,947	0	0	0	0	1,947	Paved parking
89,974	0	0	0	0	89,974	Paved roads w/curbs & sewers
44,000	0	0	0	0	44,000	Roofs
1,724,339	0	0	0	0	1,724,339	Woods, Good
2,264,103	0	0	0	0	2,264,103	TOTAL AREA

# Ground Covers (all nodes)

<b>22-243 Post Development - Rev 2</b> Prepared by Landtech Consultants, Inc. HydroCAD® 10.00-26 s/n 00798 © 2020 Hydro	<i>Type III 24-hr 2 Year Rainfall=3.01"</i> Printed 1/14/2025 DCAD Software Solutions LLC Page 5
Runoff by SCS TR	-20.00 hrs, dt=0.05 hrs, 301 points -20 method, UH=SCS, Weighted-CN ans method . Pond routing by Stor-Ind method
Subcatchment PR-1A: South of stream Flow Length=1,03	Runoff Area=947,061 sf 0.21% Impervious Runoff Depth=0.00" 5' Slope=0.0020 '/' Tc=104.9 min CN=32 Runoff=0.00 cfs 0 cf
Subcatchment PR-1B: North of stream	Runoff Area=747,420 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=860' Tc=43.9 min CN=33 Runoff=0.00 cfs 0 cf
Subcatchment PR-1C: Jenkins - entrance	Runoff Area=32,617 sf 41.40% Impervious Runoff Depth>1.16" Tc=5.0 min CN=80 Runoff=1.09 cfs 3,152 cf
Subcatchment PR-1D: Jenkins - Middle	Runoff Area=104,358 sf 41.83% Impervious Runoff Depth>1.22" Tc=5.0 min CN=81 Runoff=3.68 cfs 10,617 cf
Subcatchment PR-1E: Gratuity Rd - middle	Runoff Area=45,685 sf 37.54% Impervious Runoff Depth>1.10" Tc=5.0 min CN=79 Runoff=1.45 cfs 4,189 cf
Subcatchment PR-1F: Gratuity Rd -	Runoff Area=77,252 sf 45.33% Impervious Runoff Depth>1.28" Tc=5.0 min CN=82 Runoff=2.87 cfs 8,266 cf
Subcatchment PR-1G: (new Subcat) Flow Length=3	Runoff Area=131,193 sf 0.00% Impervious Runoff Depth=0.00" 00' Slope=0.0100 '/' Tc=24.8 min CN=40 Runoff=0.00 cfs 0 cf
Subcatchment PR-1H: (new Subcat) Flow Length=3	Runoff Area=107,768 sf 0.00% Impervious Runoff Depth>0.00" 00' Slope=0.0100 '/' Tc=24.8 min CN=41 Runoff=0.00 cfs 0 cf
Subcatchment PR-1I:	Runoff Area=13,678 sf 0.00% Impervious Runoff Depth>0.14" Tc=5.0 min CN=54 Runoff=0.02 cfs 162 cf
Subcatchment PR-1K:	Runoff Area=16,989 sf 0.00% Impervious Runoff Depth>0.00" Tc=5.0 min CN=43 Runoff=0.00 cfs 5 cf
	eRunoff Area=18,114 sf 14.80% Impervious Runoff Depth>0.74" Slope=0.0100 '/' Tc=9.8 min CN=72 Runoff=0.31 cfs 1,110 cf
Subcatchment Unit 1: Unit A - Birch	Runoff Area=1,785 sf 100.00% Impervious Runoff Depth>2.60" Tc=5.0 min CN=98 Runoff=0.12 cfs 387 cf
Subcatchment Unit 10: Unit C - Hickory	Runoff Area=1,857 sf 100.00% Impervious Runoff Depth>2.60" Tc=5.0 min CN=98 Runoff=0.13 cfs 402 cf
Subcatchment Unit 11: Unit B - Hemlock	Runoff Area=1,850 sf 100.00% Impervious Runoff Depth>2.60" Tc=5.0 min CN=98 Runoff=0.12 cfs 401 cf
Subcatchment Unit 12: Unit C - Hickory	Runoff Area=1,857 sf 100.00% Impervious Runoff Depth>2.60" Tc=5.0 min CN=98 Runoff=0.13 cfs 402 cf
Subcatchment Unit 2: Unit B - Hemlock	Runoff Area=1,850 sf 100.00% Impervious Runoff Depth>2.60" Tc=5.0 min CN=98 Runoff=0.12 cfs 401 cf

<b>22-243 Post Development - Rev 2</b> Prepared by Landtech Consultants, In HydroCAD® 10.00-26 s/n 00798 © 2020 F	лс.	Year Rainfall=3.01" Printed 1/14/2025 Page 6
Subcatchment Unit 3: Unit C - Hickory	Runoff Area=1,857 sf 100.00% Impervious Tc=5.0 min CN=98 F	
Subcatchment Unit 4: Unit A - Birch	Runoff Area=1,785 sf 100.00% Imperviou Tc=5.0 min CN=98 F	
Subcatchment Unit 5: Unit B - Hemlock	Runoff Area=1,850 sf 100.00% Impervious Tc=5.0 min CN=98 F	
Subcatchment Unit 6: Unit C - Hickory	Runoff Area=1,857 sf 100.00% Impervious Tc=5.0 min CN=98 F	
Subcatchment Unit 7: Unit A - Birch	Runoff Area=1,785 sf 100.00% Imperviou Tc=5.0 min CN=98 F	
Subcatchment Unit 8: Unit A - Birch	Runoff Area=1,785 sf 100.00% Imperviou Tc=5.0 min CN=98 F	
Subcatchment Unit 9: Unit B - Hemlock	Runoff Area=1,850 sf 100.00% Impervious Tc=5.0 min CN=98 F	
Reach DP-1: Stream		Inflow=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf
Reach DP-2: Gratuity Road		flow=0.31 cfs 1,110 cf flow=0.31 cfs 1,110 cf
Pond DE-1: Drip Edge	Peak Elev=0.13' Storage=0.000 af ded=0.07 cfs 387 cf Primary=0.00 cfs 0 cf O	
Pond DE-10: Drip Edge	Peak Elev=0.15' Storage=0.001 af rded=0.07 cfs 402 cf Primary=0.00 cfs 0 cf O	
Pond DE-11: Drip Edge	Peak Elev=0.19' Storage=0.001 af rded=0.07 cfs 401 cf Primary=0.00 cfs 0 cf O	
Pond DE-12: Drip Edge	Peak Elev=0.15' Storage=0.001 af rded=0.07 cfs 402 cf Primary=0.00 cfs 0 cf O	
Pond DE-2: Drip Edge	Peak Elev=0.24' Storage=0.001 af rded=0.06 cfs 401 cf Primary=0.00 cfs 0 cf O	
Pond DE-3: Drip Edge	Peak Elev=0.21' Storage=0.001 af rded=0.07 cfs 402 cf Primary=0.00 cfs 0 cf O	
Pond DE-4: Drip Edge	Peak Elev=0.28' Storage=0.001 af rded=0.06 cfs  387 cf  Primary=0.00 cfs  0 cf   O	
Pond DE-5: Drip Edge	Peak Elev=0.19' Storage=0.001 af	Inflow=0.12 cfs 401 cf

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Pond DE-6: Drip Edge	Peak Elev=0.15' Storage=0.001 af Inflow=0.13 cfs 402 cf Discarded=0.07 cfs 402 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 402 cf
Pond DE-7: Drip Edge	Peak Elev=0.22' Storage=0.001 af Inflow=0.12 cfs 387 cf Discarded=0.06 cfs 387 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 387 cf
Pond DE-8: Drip Edge	Peak Elev=0.13' Storage=0.000 af Inflow=0.12 cfs 387 cf Discarded=0.07 cfs 387 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 387 cf
Pond DE-9: Drip Edge	Peak Elev=0.29' Storage=0.001 af Inflow=0.12 cfs 401 cf Discarded=0.06 cfs 401 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 401 cf
Pond GD-1: Ground Depression	Peak Elev=211.60' Storage=0 cf Inflow=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf
Pond GD-2: Ground Depression	Peak Elev=210.60' Storage=0 cf Inflow=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf
Pond GD-3: Ground Depression	Peak Elev=213.60' Storage=1 cf Inflow=0.02 cfs 162 cf Outflow=0.02 cfs 162 cf
Pond GD-4: Ground Depression	Peak Elev=213.60' Storage=0 cf Inflow=0.00 cfs 5 cf Outflow=0.00 cfs 5 cf
Pond IB-1: Infiltration Basin #1 D	Peak Elev=211.03' Storage=139 cf Inflow=1.09 cfs 3,152 cf iscarded=0.86 cfs 3,150 cf Primary=0.00 cfs 0 cf Outflow=0.86 cfs 3,150 cf
Pond IB-2: Infiltration Basin #2	Peak Elev=212.18' Storage=1,464 cf Inflow=3.68 cfs 10,617 cf arded=1.55 cfs 10,606 cf Primary=0.00 cfs 0 cf Outflow=1.55 cfs 10,606 cf
Pond IB-3: Infiltration Basin #3	Peak Elev=213.52' Storage=160 cf Inflow=1.45 cfs 4,189 cf iscarded=1.26 cfs 4,186 cf Primary=0.00 cfs 0 cf Outflow=1.26 cfs 4,186 cf
Pond IB-4: Infiltration Basin #4 D	Peak Elev=211.64' Storage=962 cf Inflow=2.87 cfs 8,266 cf iscarded=1.34 cfs 8,259 cf Primary=0.00 cfs 0 cf Outflow=1.34 cfs 8,259 cf
Total Runoff Area = 2	2,264,103 sf Runoff Volume = 32,261 cf Average Runoff Depth = 0.17" 94.00% Pervious = 2,128,182 sf 6.00% Impervious = 135,921 sf

# Summary for Subcatchment PR-1A: South of stream

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 5.00 hrs, Volume=

0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

_	A	rea (sf)	CN	Description						
-	8	97,676	30	Woods, Go	od, HSG A					
		47,438	68	<50% Gras	s cover, Po	or, HSG A				
		1,947	98	Paved park	ing, HSG A					
-	9	47,061	32							
	9	45,114		99.79% Pei	rvious Area					
		1,947		0.21% Impervious Area						
	Тс	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	) (ft/sec)	(cfs)					
	31.5	50	0.0020	0.03		Sheet Flow, A-B				
						Woods: Light underbrush n= 0.400 P2= 3.10"				
	73.4	985	0.0020	0.22		Shallow Concentrated Flow, B-C				
_						Woodland Kv= 5.0 fps				
	1010	4 005	Tatal							

104.9 1,035 Total

# Summary for Subcatchment PR-1B: North of stream

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

_	А	rea (sf)	CN E	escription					
	6	46,242	30 V	Voods, Go	od, HSG A				
		67,899	68 <	50% Grass	s cover, Po	or, HSG A			
		33,279	30 N	30 Meadow, non-grazed, HSG A					
	7	47,420	33 V	33 Weighted Average					
	7	47,420	1	00.00% Pe	ervious Are	а			
	Тс	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.7	50	0.0200	0.15		Sheet Flow, A-B			
						Grass: Short n= 0.150 P2= 3.10"			
	38.2	810	0.0050	0.35		Shallow Concentrated Flow, B-C			
_						Woodland Kv= 5.0 fps			
	43.9	860	Total						

#### Summary for Subcatchment PR-1C: Jenkins - entrance

[49] Hint: Tc<2dt may require smaller dt

Runoff = 1.09 cfs @ 12.08 hrs, Volume= 3,152 cf, Depth> 1.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

A	rea (sf)	CN	Description		
	9,869	98	Paved road	s w/curbs &	& sewers, HSG A
	19,113	68	<50% Gras	s cover, Po	oor, HSG A
	3,635	98	Roofs, HSC	βA	
	32,617	80	Weighted A	verage	
	19,113		58.60% Pe	vious Area	а
	13,504		41.40% Imp	pervious Are	rea
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	
5.0					Direct Entry,

#### Summary for Subcatchment PR-1D: Jenkins - Middle

[49] Hint: Tc<2dt may require smaller dt

Runoff = 3.68 cfs @ 12.08 hrs, Volume= 10,617 cf, Depth> 1.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

Area (sf)	CN	Description					
34,458	98	Paved road	s w/curbs &	& sewers, HSG A			
60,702	68	<50% Gras	s cover, Po	bor, HSG A			
9,198	98	Roofs, HSC	βA				
104,358	81	Weighted A	verage				
60,702		58.17% Per	rvious Area	l			
43,656		41.83% Imp	pervious Ar	ea			
Tc Length	Slop	,	Capacity	Description			
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)				
5.0				Direct Entry,			

#### Summary for Subcatchment PR-1E: Gratuity Rd - middle near entrance

[49] Hint: Tc<2dt may require smaller dt

Runoff = 1.45 cfs @ 12.08 hrs, Volume= 4,189 cf, Depth> 1.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01" 22-243 Post Development - Rev 2

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Ar	ea (sf)	CN I	CN Description				
	15,364	98 I	Paved road	s w/curbs &	& sewers, HSG A		
	28,536	68 ·	<50% Gras	s cover, Po	bor, HSG A		
	1,785	98 I	Roofs, HSG	β A			
-	45,685	79	Neighted A	verage			
	28,536	(	62.46% Per	vious Area	3		
	17,149		37.54% Imp	pervious Ar	rea		
То	Longth	Slong	Volocity	Capacity	Description		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	(ieel)	(1011)	(INSEC)	(015)	Dive et Futur		
5.0					Direct Entry,		

# Summary for Subcatchment PR-1F: Gratuity Rd - cul-de-sac

[49] Hint: Tc<2dt may require smaller dt

Runoff = 2.87 cfs @ 12.08 hrs, Volume= 8,266 cf, Depth> 1.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

A	rea (sf)	CN	Description					
	27,603	98	Paved road	s w/curbs &	& sewers, HSG A			
	42,235	68	<50% Gras	s cover, Po	oor, HSG A			
	7,414	98	Roofs, HSC	β A				
	77,252	82	Weighted A	verage				
	42,235		54.67% Per	vious Area	а			
	35,017		45.33% Imp	pervious Ar	rea			
_				<b>•</b> •	<b>_</b>			
Tc	Length	Slope	,	Capacity	1			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

#### Summary for Subcatchment PR-1G: (new Subcat)

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

Α	rea (sf)	CN	Description
	33,650	68	<50% Grass cover, Poor, HSG A
	97,543	30	Woods, Good, HSG A
	131,193	40	Weighted Average
	131,193		100.00% Pervious Area

Prepare	d by Lar	ndtech C	onsultant 0798 © 202	s, Inc.	Type III 24-hr 2 Year Rainfall=3.01"Printed 1/14/2025D Software Solutions LLCPage 11
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.5	50	0.0100	0.05		Sheet Flow,
8.3	250	0.0100	0.50		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
24.8	300	Total			
	Summary for Subcatchment PR-1H: (new Subcat)				
[73] War	ning: Pea	ak may fa	ll outside t	ime span	
Runoff	=	0.00 cfs	s@ 20.0	0 hrs, Volu	me= 0 cf, Depth> 0.00"
			nod, UH=S fall=3.01"	CS, Weigh	ted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
A	rea (sf)	CN D	escription		
	29,892			s cover, Po	
	77,876			od, HSG A	
	07,768 07,768		/eighted A 00.00% Pe	verage ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.5	50	0.0100	0.05		Sheet Flow,
8.3	250	0.0100	0.50		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
24.8	300	Total			
			Sum	mary for	Subcatchment PR-1I:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.02 cfs @ 12.38 hrs, Volume= 162 cf, Depth> 0.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

Area (sf)	CN	Description
8,676	68	<50% Grass cover, Poor, HSG A
5,002	30	Woods, Good, HSG A
13,678 13,678	54	Weighted Average 100.00% Pervious Area

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
5.0 Direct Entry,	
Summary for Subcatchment	PR-1K:
[49] Hint: Tc<2dt may require smaller dt [73] Warning: Peak may fall outside time span	
Runoff = 0.00 cfs @ 20.00 hrs, Volume=	5 cf, Depth> 0.00"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Spa Type III 24-hr 2 Year Rainfall=3.01"	n= 5.00-20.00 hrs, dt= 0.05 hrs
Area (sf) CN Description	
5,678 68 <50% Grass cover, Poor, HSG A	
11,311 30 Meadow, non-grazed, HSG A	
16,98943Weighted Average16,989100.00% Pervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
5.0 Direct Entry,	
Summary for Subcatchment PR-2: Grat	uity Rd - entrance
Runoff = 0.31 cfs @ 12.16 hrs, Volume= 1,11	10 cf, Depth> 0.74"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Spa Type III 24-hr 2 Year Rainfall=3.01"	n= 5.00-20.00 hrs, dt= 0.05 hrs
Area (sf) CN Description	
2,680 98 Paved roads w/curbs & sewers, HSG A	
15,434 68 <50% Grass cover, Poor, HSG A	
18,114 72 Weighted Average	
15,434 85.20% Pervious Area	

	2,680	1	4.80% Imp	ervious Are	ea		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
7.5	50	0.0100	0.11	\$ <i>L</i>	Sheet Flow,	 D0- 0 40"	

 
 (min)
 (feet)
 (ft/ft)
 (ft/sec)
 (cfs)

 7.5
 50
 0.0100
 0.11
 Sheet Flow, Grass: Short n= 0.150 P2= 3.10"

 2.3
 98
 0.0100
 0.70
 Shallow Concentrated Flow, Short Grass Pasture

 9.8
 148
 Total

# Summary for Subcatchment Unit 1: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.12 cfs @ 12.07 hrs, Volume= 387 cf, Depth> 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

Ar	ea (sf)	CN	Description				
	1,785	98	Roofs, HSG A				
	1,785		100.00% In	npervious A	Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

## Summary for Subcatchment Unit 10: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff	=	0.13 cfs @	12.07 hrs, Volume=	402 cf, Depth> 2.60"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

A	rea (sf)	CN	Description		
	1,857	98	Roofs, HSG	βA	
	1,857		100.00% In	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
5.0					Direct Entry,

#### Summary for Subcatchment Unit 11: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.12 cfs @ 12.07 hrs, Volume= 401 cf, Depth> 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

 Area (sf)	CN	Description
1,850	98	Roofs, HSG A
 1,850		100.00% Impervious Area

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
5.0 Direct Entry,
Summary for Subcatchment Unit 12: Unit C - Hickory
[49] Hint: Tc<2dt may require smaller dt
Runoff = 0.13 cfs @ 12.07 hrs, Volume= 402 cf, Depth> 2.60"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"
Area (sf) CN Description
1,857 98 Roofs, HSG A
1,857 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
5.0 Direct Entry,
Summary for Subcatchment Unit 2: Unit B - Hemlock
Summary for Subcatchment Onit 2. Onit D - Hennock
[49] Hint: Tc<2dt may require smaller dt
Runoff = 0.12 cfs @ 12.07 hrs, Volume= 401 cf, Depth> 2.60"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"
Area (sf) CN Description
1,850 98 Roofs, HSG A
1,850 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
5.0 Direct Entry,
Summary for Subcatchment Unit 3: Unit C - Hickory
[49] Hint: Tc<2dt may require smaller dt
Runoff = 0.13 cfs @ 12.07 hrs, Volume= 402 cf, Depth> 2.60"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

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A	rea (sf)	CN	Description		
	1,857	98	Roofs, HSG	β A	
	1,857		100.00% In	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	
5.0					Direct Entry,

### Summary for Subcatchment Unit 4: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.12 cfs @ 12.07 hrs, Volume= 387 cf, Depth> 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

Are	ea (sf)	CN	Description		
	1,785	98	Roofs, HSG	βA	
	1,785		100.00% In	npervious A	rea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

# Summary for Subcatchment Unit 5: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.12 cfs @ 12.07 hrs, Volume= 401 cf, Depth> 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

A	rea (sf)	CN	Description		
	1,850	98	Roofs, HSG	ΞA	
	1,850		100.00% In	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
5.0					Direct Entry,

# Summary for Subcatchment Unit 6: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.13 cfs @ 12.07 hrs, Volume= 402 cf, Depth> 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

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A	rea (sf)	CN	Description		
	1,857	98	Roofs, HSG	βA	
	1,857		100.00% Im	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description
5.0					Direct Entry,

#### Summary for Subcatchment Unit 7: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.12 cfs @ 12.07 hrs, Volume= 387 cf, Depth> 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

A	rea (sf)	CN Description					
	1,785	98	98 Roofs, HSG A				
	1,785	100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description		
5.0					Direct Entry,		

# Summary for Subcatchment Unit 8: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.12 cfs @ 12.07 hrs, Volume= 387 cf, Depth> 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

Area (sf)	CN Descriptio	n	
1,785	98 Roofs, HS	GA	
1,785	100.00% I	mpervious A	Area
Tc Length (min) (feet)	Slope Velocity (ft/ft) (ft/sec		Description
5.0			Direct Entry,

# Summary for Subcatchment Unit 9: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.12 cfs @ 12.07 hrs, Volume=

401 cf, Depth> 2.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2 Year Rainfall=3.01"

A	rea (sf)	CN I	Description				
	1,850	98	98 Roofs, HSG A				
	1,850		100.00% In	npervious A	vrea		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

### Summary for Reach DP-1: Stream

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	ea =	1,694,481 sf,	0.11% Impervious,	Inflow Depth = 0.00"	for 2 Year event
Inflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf	
Outflow	=	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

# Summary for Reach DP-2: Gratuity Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	18,114 sf,	14.80% Impervious,	Inflow Depth > 0.74	for 2 Year event
Inflow	=	0.31 cfs @	12.16 hrs, Volume=	1,110 cf	
Outflow	=	0.31 cfs @	12.16 hrs, Volume=	1,110 cf, Att	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

# Summary for Pond DE-1: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage

Inflow Area =	1,785 sf,100.00% Impervious,	Inflow Depth > 2.60" for 2 Year event
Inflow =	0.12 cfs @ 12.07 hrs, Volume=	387 cf
Outflow =	0.07 cfs @ 12.00 hrs, Volume=	387 cf, Atten= 38%, Lag= 0.0 min
Discarded =	0.07 cfs @ 12.00 hrs, Volume=	387 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.13' @ 12.17 hrs Surf.Area= 0.009 ac Storage= 0.000 af

Plug-Flow detention time= 1.3 min calculated for 387 cf (100% of inflow) Center-of-Mass det. time= 1.2 min (739.6 - 738.4)

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Volume	Invert	Avail.Storage	Storage Description		
#1	0.00'	0.007 af	3.00'W x 130.00'L x 2.00'H Prismatoid		
			0.018 af Overall x 40.0% Voids		
Device	Routing		utlet Devices		
#1	Discarded		270 in/hr Exfiltration over Surface area		
#2	Primary		<b>2.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00		
			50 3.00 3.50		
			oef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 85 3.07 3.20 3.32		
		Max=0.07 cfs @ xfiltration Contro	) 12.00 hrs HW=0.02' (Free Discharge) Is 0.07 cfs)		
			6.00 hrs HW=0.00' (Free Discharge) <b>eir</b> ( Controls 0.00 cfs)		
		Sum	mary for Pond DE-10: Drip Edge		
		nflow requires ea #2 is above defi			
Inflow A	rea =	1,857 sf,100.0	00% Impervious, Inflow Depth > 2.60" for 2 Year event		
Inflow		.13 cfs @ 12.07			
Outflow Discarde		0.07 cfs @ 12.00 0.07 cfs @ 12.00			
Primary		.00 cfs @ 5.00			
	Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.15' @ 12.18 hrs Surf.Area= 0.009 ac Storage= 0.001 af				
Plug-Flc	w detention	time= (not calcul	ated: outflow precedes inflow)		
		time= 1.3 min ( 7			
Volume	Invert	Avail.Storage	Storage Description		
#1	0.00'	0	3.00'W x 130.00'L x 2.00'H Prismatoid		
			0.018 af Overall x 40.0% Voids		
Device	Routing	Invert O	utlet Devices		
#1	Discarded	0.00 <b>' 8.</b>	270 in/hr Exfiltration over Surface area		
#2	Primary		2.0' long x 2.0' breadth Broad-Crested Rectangular Weir		
			ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 50 3.00 3.50		
			oef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88		
			85 3.07 3.20 3.32		

**Discarded OutFlow** Max=0.07 cfs @ 12.00 hrs HW=0.02' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond DE-11: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage

Inflow Area =	1,850 sf,100.00% Impervious,	Inflow Depth > 2.60" for 2 Year event
Inflow =	0.12 cfs @ 12.07 hrs, Volume=	401 cf
Outflow =	0.07 cfs @ 12.00 hrs, Volume=	401 cf, Atten= 45%, Lag= 0.0 min
Discarded =	0.07 cfs @ 12.00 hrs, Volume=	401 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.19' @ 12.19 hrs Surf.Area= 0.008 ac Storage= 0.001 af

Plug-Flow detention time= 1.8 min calculated for 401 cf (100% of inflow) Center-of-Mass det. time= 1.6 min (740.1 - 738.4)

Volume	Invert	Avail.Storage	e Storage Description
#1	0.00'	0.007 a	af <b>3.00'W x 120.00'L x 2.00'H Prismatoid</b> 0.017 af Overall x 40.0% Voids
Device	Routing	Invert (	Outlet Devices
#1	Discarded	0.00' 8	3.270 in/hr Exfiltration over Surface area
#2	Primary		72.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50
		(	Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.07 cfs @ 12.00 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond DE-12: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

22-243 Post Development - Rev 2Type III 24-hr2 Year Rainfall=3.01"Prepared by Landtech Consultants, Inc.Printed 1/14/2025HydroCAD® 10.00-26 s/n 00798 © 2020 HydroCAD Software Solutions LLCPage 20				
Inflow Area = $1,857  ext{ sf},100.00\%$ Impervious, Inflow Depth > $2.60"$ for 2 Year eventInflow = $0.13  ext{ cfs}$ $12.07  ext{ hrs}$ , Volume= $402  ext{ cf}$ Outflow = $0.07  ext{ cfs}$ $12.00  ext{ hrs}$ , Volume= $402  ext{ cf}$ , Atten= $40\%$ , Lag= $0.0  ext{ min}$ Discarded = $0.07  ext{ cfs}$ $12.00  ext{ hrs}$ , Volume= $402  ext{ cf}$ Primary = $0.00  ext{ cfs}$ $5.00  ext{ hrs}$ , Volume= $0  ext{ cf}$				
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.15' @ 12.18 hrs Surf.Area= 0.009 ac Storage= 0.001 af				
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 1.3 min ( 739.8 - 738.4 )				
VolumeInvertAvail.StorageStorage Description#10.00'0.007 af <b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids				
Device         Routing         Invert         Outlet Devices           #1         Discarded         0.00'         8.270 in/hr Exfiltration over Surface area           #2         Primary         2.00'         72.0' long x 2.0' breadth Broad-Crested Rectangular Weir           Head (feet)         0.20         0.40         0.60         0.80         1.00         1.20         1.40         1.60         1.80         2.00'           2.50         3.00         3.50         Coef. (English)         2.54         2.61         2.60         2.66         2.70         2.77         2.89         2.88           2.85         3.07         3.20         3.32         3.32         3.32         3.32				
<b>Discarded OutFlow</b> Max=0.07 cfs @ 12.00 hrs HW=0.02' (Free Discharge) <b>1=Exfiltration</b> (Exfiltration Controls 0.07 cfs)				
Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) <sup>↑</sup> —2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)				
Summary for Pond DE-2: Drip Edge				
[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage				
Inflow Area =       1,850 sf,100.00% Impervious, Inflow Depth > 2.60" for 2 Year event         Inflow =       0.12 cfs @ 12.07 hrs, Volume=       401 cf         Outflow =       0.06 cfs @ 12.00 hrs, Volume=       401 cf, Atten= 49%, Lag= 0.0 min         Discarded =       0.06 cfs @ 12.00 hrs, Volume=       401 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf				
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.24' @ 12.21 hrs Surf.Area= 0.008 ac Storage= 0.001 af				
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 2.1 min(740.5 - 738.4)				
VolumeInvertAvail.StorageStorage Description#10.00'0.006 af <b>3.00'W x 110.00'L x 2.00'H Prismatoid</b> 0.015 af Overall x 40.0% Voids				

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Device	Routing	Invert	Outlet Devices		
#1 #2	Discarded Primary	0.00'	8.270 in/hr Exfiltration over Surface area 72.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32		
	ed OutFlow Ma filtration (Exfil		s @ 12.00 hrs HW=0.04' (Free Discharge) htrols 0.06 cfs)		
			⊉ 5.00 hrs HW=0.00' (Free Discharge) • <b>Weir</b> (Controls 0.00 cfs)		
		Si	ummary for Pond DE-3: Drip Edge		
			s earlier time span defined storage		
Peak Ele Plug-Flo	= 0.13 = 0.07 ed = 0.07 = 0.00 by Stor-Ind met ev= 0.21' @ 12. w detention tim	3 cfs @ 12 7 cfs @ 12 7 cfs @ 12 0 cfs @ 12 10 cfs @ 1	00.00% Impervious, Inflow Depth > 2.60" for 2 Year event 2.07 hrs, Volume= 402 cf 2.00 hrs, Volume= 402 cf, Atten= 47%, Lag= 0.0 min 2.00 hrs, Volume= 402 cf 5.00 hrs, Volume= 0 cf Span= 5.00-20.00 hrs, dt= 0.05 hrs urf.Area= 0.008 ac Storage= 0.001 af culated: outflow precedes inflow)		
			(740.3 - 738.4)		
<u>Volume</u> #1	<u>Invert</u> 0.00'	<u>Avail.Stora</u> 0.006	age Storage Description 6 af <b>3.00'W x 115.00'L x 2.00'H Prismatoid</b> 0.016 af Overall x 40.0% Voids		
Device	Routing		Outlet Devices		
#1 #2	Discarded Primary	0.00' 2.00'	8.270 in/hr Exfiltration over Surface area 72.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32		
	<b>Discarded OutFlow</b> Max=0.07 cfs @ 12.00 hrs HW=0.03' (Free Discharge) <b>1=Exfiltration</b> (Exfiltration Controls 0.07 cfs)				
Primary	OutFlow Max	=0.00 cfs (	⊅ 5.00 brs. HW=0.00' (Free Discharge)		

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond DE-4: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage

Inflow Area =	1,785 sf,100.00% Impervious,	Inflow Depth > 2.60" for 2 Year event
Inflow =	0.12 cfs @ 12.07 hrs, Volume=	387 cf
Outflow =	0.06 cfs @ 12.00 hrs, Volume=	387 cf, Atten= 52%, Lag= 0.0 min
Discarded =	0.06 cfs @ 12.00 hrs, Volume=	387 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.28' @ 12.22 hrs Surf.Area= 0.007 ac Storage= 0.001 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 2.4 min (740.9 - 738.4)

Volume	Invert	Avail.Storag	e Storage Description
#1	0.00'	0.006 a	af <b>3.00'W x 100.00'L x 2.00'H Prismatoid</b> 0.014 af Overall x 40.0% Voids
Device	Routing	Invert (	Outlet Devices
#1	Discarded	0.00'	8.270 in/hr Exfiltration over Surface area
#2	Primary		<b>72.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.06 cfs @ 12.00 hrs HW=0.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond DE-5: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage

Inflow Area =	1,850 sf,100.00% Impervious,	Inflow Depth > 2.60" for 2 Year event
Inflow =	0.12 cfs @ 12.07 hrs, Volume=	401 cf
Outflow =	0.07 cfs @ 12.00 hrs, Volume=	401 cf, Atten= 45%, Lag= 0.0 min
Discarded =	0.07 cfs @ 12.00 hrs, Volume=	401 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.19' @ 12.19 hrs Surf.Area= 0.008 ac Storage= 0.001 af

Plug-Flow detention time= 1.8 min calculated for 401 cf (100% of inflow) Center-of-Mass det. time= 1.6 min (740.1 - 738.4)

Type III 24-hr 2 Year Rainfall=3.01" Prepared by Landtech Consultants, Inc. HydroCAD® 10.00-26 s/n 00798 © 2020 HydroCAD Software Solutions LLC Printed 1/14/2025 Page 23

Volume	Invert	Avail.Storage	Storage Description	
#1	0.00'	0.007 af	3.00'W x 120.00'L x 2.00'H Prismatoid	
			0.017 af Overall x 40.0% Voids	
Device	Routing	Invert Outlet Devices		
#1 #2	Discarded Primary	2.00' <b>72</b> He 2.5 Co		
		Max=0.07 cfs @ filtration Control	2 12.00 hrs HW=0.03' (Free Discharge) s 0.07 cfs)	
			.00 hrs HW=0.00' (Free Discharge) eir (Controls 0.00 cfs)	
Summary for Pond DE-6: Drip Edge				
		flow requires ea #2 is above defi		
Inflow A Inflow Outflow Discarde Primary	= 0.7 = 0.0 = 0.0	13 cfs @  12.07 )7 cfs @  12.00 )7 cfs @  12.00	hrs, Volume= 402 cf, Atten= 40%, Lag= 0.0 min	
			an= 5.00-20.00 hrs, dt= 0.05 hrs \rea= 0.009 ac Storage= 0.001 af	
		me= (not calcula me= 1.3 min ( 7	ated: outflow precedes inflow) 39.8 - 738.4)	
Volume	Invert	Avail.Storage	Storage Description	
#1	0.00'	0.007 af	<b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids	
Device	Routing	Invert Ou	utlet Devices	
#1 #2	Discarded Primary	2.00' <b>72</b> He 2.5	<b>270 in/hr Exfiltration over Surface area</b> <b>.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 50 3.00 3.50 eaf (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.80 2.89	

Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.07 cfs @ 12.00 hrs HW=0.02' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond DE-7: Drip Edge

[82] Warning: Early inflow requires earlier time span[92] Warning: Device #2 is above defined storage

Inflow Area =	1,785 sf,100.00% Impervious,	Inflow Depth > 2.60" for 2 Year event
Inflow =	0.12 cfs @ 12.07 hrs, Volume=	387 cf
Outflow =	0.06 cfs @ 12.00 hrs, Volume=	387 cf, Atten= 47%, Lag= 0.0 min
Discarded =	0.06 cfs @ 12.00 hrs, Volume=	387 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.22' @ 12.20 hrs Surf.Area= 0.008 ac Storage= 0.001 af

Plug-Flow detention time= 2.0 min calculated for 387 cf (100% of inflow) Center-of-Mass det. time= 1.9 min (740.3 - 738.4)

Volume	Invert	Avail.Storag	ge Storage Description
#1	0.00'	0.006	af <b>3.00'W x 110.00'L x 2.00'H Prismatoid</b> 0.015 af Overall x 40.0% Voids
Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	8.270 in/hr Exfiltration over Surface area
#2	Primary		72.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.06 cfs @ 12.00 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond DE-8: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

22-243 Post Development - Rev 2Type III 24-hr2 Year Rainfall=3.01"Prepared by Landtech Consultants, Inc.Printed 1/14/2025HydroCAD® 10.00-26 s/n 00798 © 2020 HydroCAD Software Solutions LLCPage 25
Inflow Area =       1,785 sf,100.00% Impervious, Inflow Depth > 2.60" for 2 Year event         Inflow =       0.12 cfs @ 12.07 hrs, Volume=       387 cf         Outflow =       0.07 cfs @ 12.00 hrs, Volume=       387 cf, Atten= 38%, Lag= 0.0 min         Discarded =       0.07 cfs @ 12.00 hrs, Volume=       387 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.13' @ 12.17 hrs Surf.Area= 0.009 ac Storage= 0.000 af
Plug-Flow detention time= 1.3 min calculated for 387 cf (100% of inflow) Center-of-Mass det. time= 1.2 min ( 739.6 - 738.4 )
VolumeInvertAvail.StorageStorage Description#10.00'0.007 af <b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids
Device         Routing         Invert         Outlet Devices           #1         Discarded         0.00'         8.270 in/hr Exfiltration over Surface area           #2         Primary         2.00'         72.0' long x 2.0' breadth Broad-Crested Rectangular Weir           Head (feet)         0.20         0.40         0.60         0.80         1.00         1.20         1.40         1.60         1.80         2.00'           2.50         3.00         3.50         Coef. (English)         2.54         2.61         2.60         2.66         2.70         2.77         2.89         2.88
Discarded OutFlow Max=0.07 cfs @ 12.00 hrs HW=0.02' (Free Discharge) ←1=Exfiltration (Exfiltration Controls 0.07 cfs)
Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge)
Summary for Pond DE-9: Drip Edge
[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage
Inflow Area =       1,850 sf,100.00% Impervious, Inflow Depth > 2.60" for 2 Year event         Inflow =       0.12 cfs @ 12.07 hrs, Volume=       401 cf         Outflow =       0.06 cfs @ 12.00 hrs, Volume=       401 cf, Atten= 53%, Lag= 0.0 min         Discarded =       0.06 cfs @ 12.00 hrs, Volume=       401 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.29' @ 12.22 hrs Surf.Area= 0.007 ac Storage= 0.001 af
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 2.5 min(741.0 - 738.4)
VolumeInvertAvail.StorageStorage Description#10.00'0.006 af <b>3.00'W x 102.00'L x 2.00'H Prismatoid</b> 0.014 af Overall x 40.0% Voids

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Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	8.270 in/hr Exfiltration over Surface area
#2	Primary	2.00'	<b>72.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.06 cfs @ 12.00 hrs HW=0.04' (Free Discharge) -1=Exfiltration (Exfiltration Controls 0.06 cfs)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) **1**–2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## Summary for Pond GD-1: Ground Depression

Inflow Area =	131,193 sf,	0.00% Impervious,	Inflow Depth = 0.00" for 2 Year event
Inflow =	0.00 cfs @	5.00 hrs, Volume=	0 cf
Outflow =	0.00 cfs @	5.00 hrs, Volume=	0 cf, Atten= 0%, Lag= 0.0 min
Discarded =	0.00 cfs @	5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 211.60' @ 5.00 hrs Surf.Area= 1,514 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Sto	rage Stor	orage Description
#1	211.60'	2,90	03 cf Cus	ustom Stage Data (Prismatic) Listed below (Recalc)
Elevatio (fee		urf.Area (sq-ft)	Inc.Stor (cubic-fee	
211.6	0	1,514		0 0
212.0	0	13,000	2,90	903 2,903
Device	Routing	Invert	Outlet De	Devices
#1	Discarded	211.60'	8.270 in/l	n/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.00 cfs @ 5.00 hrs HW=211.60' (Free Discharge) **1=Exfiltration** (Passes 0.00 cfs of 0.29 cfs potential flow)

# Summary for Pond GD-2: Ground Depression

Inflow Area =	107,768 sf, 0.00% Impervious,	Inflow Depth > 0.00" for 2 Year event
Inflow =	0.00 cfs @ 20.00 hrs, Volume=	0 cf
Outflow =	0.00 cfs @ 20.00 hrs, Volume=	0 cf, Atten= 32%, Lag= 0.0 min
Discarded =	0.00 cfs @ 20.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 210.60' @ 20.00 hrs Surf.Area= 550 sf Storage= 0 cf

Plug-Flow detention time= 0.5 min calculated for 0 cf (68% of inflow) Center-of-Mass det. time= 0.0 min (1,199.9 - 1,199.9)

Volume	Invert	Avail.Sto	rage Stor	age Description	
#1	210.60'	3,54	47 cf <b>Cus</b>	tom Stage Data (P	rismatic) Listed below (Recalc)
	_				
Elevatio	n Su	ırf.Area	Inc.Store	e Cum.Store	
(feet	t)	(sq-ft)	(cubic-feet	) (cubic-feet)	
210.6	0	550		0 0	
211.0	0	2,998	71	D 710	
211.5	0	8,352	2,83	3,547	
Device	Routing	Invert	Outlet De	vices	
#1	Discarded	210.60'	8.270 in/h	r Exfiltration over	Surface area

**Discarded OutFlow** Max=0.11 cfs @ 20.00 hrs HW=210.60' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.11 cfs)

## Summary for Pond GD-3: Ground Depression

Inflow Area =	13,678 sf, 0.00% Impervious,	Inflow Depth > 0.14" for 2 Year event
Inflow =	0.02 cfs @ 12.38 hrs, Volume=	162 cf
Outflow =	0.02 cfs @ 12.39 hrs, Volume=	162 cf, Atten= 0%, Lag= 0.8 min
Discarded =	0.02 cfs @ 12.39 hrs, Volume=	162 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.60' @ 12.39 hrs Surf.Area= 357 sf Storage= 1 cf

Plug-Flow detention time= 0.8 min calculated for 162 cf (100% of inflow) Center-of-Mass det. time= 0.5 min (904.9 - 904.4)

Volume	Invert	Avail.Sto	rage Stora	age Description	
#1	213.60'	91	19 cf Cust	tom Stage Data (Prismatic) Listed below (Recalc)	
Elevatio (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)		
213.6	-	355	0		
214.0	0	660	203	3 203	
214.5	50	2,205	716	6 919	
Device	Routing	Invert	Outlet Dev	vices	
#1	Discarded	213.60'	8.270 in/h	nr Exfiltration over Surface area	
<b>.</b>		Mar. 0.07. (	0 40 00 1		

**Discarded OutFlow** Max=0.07 cfs @ 12.39 hrs HW=213.60' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

# Summary for Pond GD-4: Ground Depression

Inflow Area = Inflow = Outflow = Discarded =	= 0.00 = 0.00	cfs @ 20	0.00 hrs, 0.00 hrs,	Volume= Volume=	Inflow Dept	5 cf	for 2 Yea n= 0%, Lag	
	Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.60' @ 20.00 hrs Surf.Area= 550 sf Storage= 0 cf							
Plug-Flow de Center-of-Ma		e= 0.1 min	( 1,102.	1 - 1,101.9	)	w)		
Volume	Invert	Avail.Stor	rage St	orage Des	cription			
#1	213.60'	36	63 cf <b>C</b> l	ustom Sta	ge Data (Pris	smatic) List	ed below (F	Recalc)
Elevation	Surf.A	rea	Inc.Sto	ore	Cum.Store			
(feet)		q-ft)	(cubic-fe		cubic-feet)			
213.60		550	•	0	0			
214.00	1,	263	3	63	363			
Device Ro	uting	Invert	Outlet E	Devices				
#1 Dis	scarded	213.60'	8.270 ir	hr Exfiltr	ation over S	urface area	1	
			~ ~ ~ ~	<u> </u>			,	

**Discarded OutFlow** Max=0.11 cfs @ 20.00 hrs HW=213.60' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.11 cfs)

## Summary for Pond IB-1: Infiltration Basin #1

Inflow Area =	32,617 sf, 41.40% Impervious,	Inflow Depth > 1.16" for 2 Year event
Inflow =	1.09 cfs @ 12.08 hrs, Volume=	3,152 cf
Outflow =	0.86 cfs @ 12.14 hrs, Volume=	3,150 cf, Atten= 21%, Lag= 3.7 min
Discarded =	0.86 cfs @ 12.14 hrs, Volume=	3,150 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 211.03' @ 12.14 hrs Surf.Area= 4,510 sf Storage= 139 cf

Plug-Flow detention time= 1.8 min calculated for 3,150 cf (100% of inflow) Center-of-Mass det. time= 1.6 min (806.9 - 805.4)

Volume	Invert	Avail.Storage	Storage	Description	
#1	211.00'	11,236 cf	Custom	Stage Data (Pri	ismatic) Listed below (Recalc)
Elevation (feet)	Surf.Aı (sq		.Store c-feet)	Cum.Store (cubic-feet)	
211.00	4,4	180	0	0	
212.00	5,4	136	4,958	4,958	
213.00	7,1	119	6,278	11,236	

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Device	Routing	Invert	Outlet Devices
#1	Primary	212.00'	5.0' long x 15.0' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Discarded	211.00'	8.270 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.86 cfs @ 12.14 hrs HW=211.03' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.86 cfs)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=211.00' (Free Discharge) **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

## Summary for Pond IB-2: Infiltration Basin #2

Inflow Area =	104,358 sf, 41.83% Impervious,	Inflow Depth > 1.22" for 2 Year event
Inflow =	3.68 cfs @ 12.08 hrs, Volume=	10,617 cf
Outflow =	1.55 cfs @ 12.32 hrs, Volume=	10,606 cf, Atten= 58%, Lag= 14.4 min
Discarded =	1.55 cfs @ 12.32 hrs, Volume=	10,606 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 212.18' @ 12.32 hrs Surf.Area= 8,108 sf Storage= 1,464 cf

Plug-Flow detention time= 6.2 min calculated for 10,606 cf (100% of inflow) Center-of-Mass det. time= 5.8 min ( 808.6 - 802.8 )

Volume	Invert	Avail.Sto	rage Storage	e Description	
#1	212.00'	28,93	39 cf Custon	n Stage Data (Pr	ismatic) Listed below (Recalc)
Elevation (feet 212.00 213.00 214.00 215.00	) 0 0 0	urf.Area (sq-ft) 7,901 9,033 10,222 11,467	Inc.Store (cubic-feet) 0 8,467 9,628 10,845	Cum.Store (cubic-feet) 0 8,467 18,095 28,939	
Device	Routing	Invert	Outlet Devic	es	
	Discarded Primary	212.00' 214.00'	<b>5.0' long x</b> 1 Head (feet)	0.20 0.40 0.60	Surface area oad-Crested Rectangular Weir 0.80 1.00 1.20 1.40 1.60 70 2.64 2.63 2.64 2.64 2.63

**Discarded OutFlow** Max=1.55 cfs @ 12.32 hrs HW=212.18' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.55 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=212.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## Summary for Pond IB-3: Infiltration Basin #3

Inflow Area =	45,685 sf, 37.54% Impervious,	Inflow Depth > 1.10" for 2 Year event
Inflow =	1.45 cfs @ 12.08 hrs, Volume=	4,189 cf
Outflow =	1.26 cfs @ 12.13 hrs, Volume=	4,186 cf, Atten= 13%, Lag= 2.5 min
Discarded =	1.26 cfs @ 12.13 hrs, Volume=	4,186 cf
Primary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.52' @ 12.13 hrs Surf.Area= 6,520 sf Storage= 160 cf

Plug-Flow detention time= 1.8 min calculated for 4,172 cf (100% of inflow) Center-of-Mass det. time= 1.5 min (809.4 - 807.9)

Volume	Invert	Avail.Stor	rage Storage	Description			
#1	213.50	15,68	B1 cf Custon	Stage Data (Prismatic) Listed below (Re	calc)		
			In a Starra	Curre Store			
Elevatio		urf.Area	Inc.Store	Cum.Store			
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)			
213.5	50	6,490	0	0			
214.0	00	7,094	3,396	3,396			
214.5	50	7,710	3,701	7,097			
215.0	00	8,343	4,013	11,110			
215.8	215.50 9,940		4,571	15,681			
Device	Routing	Invert	Outlet Device	s			
#1	Discarded	213.50'	8.270 in/hr E	cfiltration over Surface area			
#2	Primary	214.50'	5.0' long x 3	0' breadth Broad-Crested Rectangular \	Neir		
	,		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00				
			```	50 4.00 4.50			
			Coef. (Englis	1) 2.44 2.58 2.68 2.67 2.65 2.64 2.64	2.68 2.68		
			· · ·	92 2.97 3.07 3.32			
Discord	Discourded OutElow, Max = 1.25 of @ 12.12 bro HIW=212.52! (Erec Discharge)						

**Discarded OutFlow** Max=1.25 cfs @ 12.13 hrs HW=213.52' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.25 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=213.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## Summary for Pond IB-4: Infiltration Basin #4

Inflow Area =	77,252 sf, 45.33% Impervious,	Inflow Depth > 1.28" for 2 Year event
Inflow =	2.87 cfs @ 12.08 hrs, Volume=	8,266 cf
Outflow =	1.34 cfs @ 12.27 hrs, Volume=	8,259 cf, Atten= 53%, Lag= 11.5 min
Discarded =	1.34 cfs @ 12.27 hrs, Volume=	8,259 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 211.64' @ 12.27 hrs Surf.Area= 7,004 sf Storage= 962 cf

Plug-Flow detention time= 4.7 min calculated for 8,232 cf (100% of inflow)

Center-of-Mass det. time= 4.3 min ( 804.5 - 800.2 )

Volume	Invert	Avail.Stor	age Storage	e Description			
#1	211.50'	211.50' 21,638		S cf Custom Stage Data (Prismatic) Listed below (Recalc)			
Elevatio	on Su	rf.Area	Inc.Store	Cum.Store			
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)			
211.5	50	6,864	0	0			
212.0	00	7,370	3,559	3,559			
213.0	213.00 9,144		8,257	11,816			
214.0	00	10,501	9,823	21,638			
Device	Routing	Invert	Outlet Device	es			
#1	Discarded	211.50'	8.270 in/hr E	Exfiltration over	Surface area		
#2	#2 Primary 213.00'		5.0' long x 15.0' breadth Broad-Crested Rectangular Weir				
			Head (feet)	0.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60		
			Coef. (Englis	sh) 2.68 2.70 2.	70 2.64 2.63 2.64 2.64 2.63		
<b>Discarded OutFlow</b> Max=1.34 cfs @ 12.27 hrs HW=211.64' (Free Discharge)							

**1=Exfiltration** (Exfiltration Controls 1.34 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=211.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

<b>22-243 Post Development - Rev 2</b> Prepared by Landtech Consultants, Inc. <u>HydroCAD® 10.00-26 s/n 00798 © 2020 Hydro</u>	<i>Type III 24-hr 10 Year Rainfall=4.44"</i> Printed 1/14/2025 DCAD Software Solutions LLC Page 32
Runoff by SCS TR	-20.00 hrs, dt=0.05 hrs, 301 points -20 method, UH=SCS, Weighted-CN ans method - Pond routing by Stor-Ind method
Subcatchment PR-1A: South of stream Flow Length=1,03	Runoff Area=947,061 sf 0.21% Impervious Runoff Depth=0.00" 5' Slope=0.0020 '/' Tc=104.9 min CN=32 Runoff=0.00 cfs 0 cf
Subcatchment PR-1B: North of stream	Runoff Area=747,420 sf 0.00% Impervious Runoff Depth>0.00" Flow Length=860' Tc=43.9 min CN=33 Runoff=0.02 cfs 69 cf
Subcatchment PR-1C: Jenkins - entrance	Runoff Area=32,617 sf 41.40% Impervious Runoff Depth>2.25" Tc=5.0 min CN=80 Runoff=2.12 cfs 6,107 cf
Subcatchment PR-1D: Jenkins - Middle	Runoff Area=104,358 sf 41.83% Impervious Runoff Depth>2.33" Tc=5.0 min CN=81 Runoff=7.02 cfs 20,262 cf
Subcatchment PR-1E: Gratuity Rd - middle	Runoff Area=45,685 sf 37.54% Impervious Runoff Depth>2.17" Tc=5.0 min CN=79 Runoff=2.86 cfs 8,242 cf
Subcatchment PR-1F: Gratuity Rd -	Runoff Area=77,252 sf 45.33% Impervious Runoff Depth>2.41" Tc=5.0 min CN=82 Runoff=5.37 cfs 15,545 cf
Subcatchment PR-1G: (new Subcat) Flow Length=300'	Runoff Area=131,193 sf 0.00% Impervious Runoff Depth>0.09" Slope=0.0100 '/' Tc=24.8 min CN=40 Runoff=0.05 cfs 1,016 cf
Subcatchment PR-1H: (new Subcat) Flow Length=300'	Runoff Area=107,768 sf 0.00% Impervious Runoff Depth>0.12" Slope=0.0100 '/' Tc=24.8 min CN=41 Runoff=0.05 cfs 1,040 cf
Subcatchment PR-1I:	Runoff Area=13,678 sf 0.00% Impervious Runoff Depth>0.58" Tc=5.0 min CN=54 Runoff=0.17 cfs 666 cf
Subcatchment PR-1K:	Runoff Area=16,989 sf 0.00% Impervious Runoff Depth>0.17" Tc=5.0 min CN=43 Runoff=0.02 cfs 242 cf
	<b>e</b> Runoff Area=18,114 sf 14.80% Impervious Runoff Depth>1.63" Slope=0.0100 '/' Tc=9.8 min CN=72 Runoff=0.74 cfs 2,463 cf
Subcatchment Unit 1: Unit A - Birch	Runoff Area=1,785 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.18 cfs 581 cf
Subcatchment Unit 10: Unit C - Hickory	Runoff Area=1,857 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.19 cfs 605 cf
Subcatchment Unit 11: Unit B - Hemlock	Runoff Area=1,850 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.19 cfs 603 cf
Subcatchment Unit 12: Unit C - Hickory	Runoff Area=1,857 sf  100.00% Impervious  Runoff Depth>3.91" Tc=5.0 min  CN=98  Runoff=0.19 cfs  605 cf
Subcatchment Unit 2: Unit B - Hemlock	Runoff Area=1,850 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.19 cfs 603 cf

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Subcatchment Unit 3: Unit C - Hid	ckory Runoff Area=1,857 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.19 cfs 605 cf
Subcatchment Unit 4: Unit A - Bir	ch Runoff Area=1,785 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.18 cfs 581 cf
Subcatchment Unit 5: Unit B - He	mlock Runoff Area=1,850 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.19 cfs 603 cf
Subcatchment Unit 6: Unit C - Hid	ckory Runoff Area=1,857 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.19 cfs 605 cf
Subcatchment Unit 7: Unit A - Bir	ch Runoff Area=1,785 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.18 cfs 581 cf
Subcatchment Unit 8: Unit A - Bir	<b>ch</b> Runoff Area=1,785 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.18 cfs 581 cf
Subcatchment Unit 9: Unit B - He	mlock Runoff Area=1,850 sf 100.00% Impervious Runoff Depth>3.91" Tc=5.0 min CN=98 Runoff=0.19 cfs 603 cf
Reach DP-1: Stream	Inflow=0.02 cfs 69 cf Outflow=0.02 cfs 69 cf
Reach DP-2: Gratuity Road	Inflow=0.74 cfs 2,463 cf Outflow=0.74 cfs 2,463 cf
Pond DE-1: Drip Edge	Peak Elev=0.39' Storage=0.001 af Inflow=0.18 cfs 581 cf Discarded=0.07 cfs 581 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 581 cf
Pond DE-10: Drip Edge	Peak Elev=0.43' Storage=0.002 af Inflow=0.19 cfs 605 cf Discarded=0.07 cfs 605 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 605 cf
Pond DE-11: Drip Edge	Peak Elev=0.52' Storage=0.002 af Inflow=0.19 cfs 603 cf Discarded=0.07 cfs 602 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 602 cf
Pond DE-12: Drip Edge	Peak Elev=0.43' Storage=0.002 af Inflow=0.19 cfs 605 cf Discarded=0.07 cfs 605 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 605 cf
Pond DE-2: Drip Edge	Peak Elev=0.64' Storage=0.002 af Inflow=0.19 cfs 603 cf Discarded=0.06 cfs 602 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 602 cf
Pond DE-3: Drip Edge	Peak Elev=0.58' Storage=0.002 af Inflow=0.19 cfs 605 cf Discarded=0.07 cfs 605 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 605 cf
Pond DE-4: Drip Edge	Peak Elev=0.73' Storage=0.002 af Inflow=0.18 cfs 581 cf Discarded=0.06 cfs 581 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 581 cf
Pond DE-5: Drip Edge	Peak Elev=0.52' Storage=0.002 af Inflow=0.19 cfs 603 cf Discarded=0.07 cfs 602 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 602 cf

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Pond DE-6: Drip Edge	Peak Elev=0.43' Storage=0.002 af Inflow=0.19 cfs 605 cf Discarded=0.07 cfs 605 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 605 cf
Pond DE-7: Drip Edge	Peak Elev=0.59' Storage=0.002 af Inflow=0.18 cfs 581 cf Discarded=0.06 cfs 581 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 581 cf
Pond DE-8: Drip Edge	Peak Elev=0.39' Storage=0.001 af Inflow=0.18 cfs 581 cf Discarded=0.07 cfs 581 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 581 cf
Pond DE-9: Drip Edge	Peak Elev=0.76' Storage=0.002 af Inflow=0.19 cfs 603 cf Discarded=0.06 cfs 602 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 602 cf
Pond GD-1: Ground Depression	Peak Elev=211.60' Storage=1 cf Inflow=0.05 cfs 1,016 cf Outflow=0.05 cfs 1,015 cf
Pond GD-2: Ground Depression	Peak Elev=210.60' Storage=2 cf Inflow=0.05 cfs 1,040 cf Outflow=0.05 cfs 1,039 cf
Pond GD-3: Ground Depression	Peak Elev=213.75' Storage=61 cf Inflow=0.17 cfs 666 cf Outflow=0.09 cfs 666 cf
Pond GD-4: Ground Depression	Peak Elev=213.60' Storage=0 cf Inflow=0.02 cfs 242 cf Outflow=0.02 cfs 242 cf
Pond IB-1: Infiltration Basin #1 Di	Peak Elev=211.18' Storage=802 cf Inflow=2.12 cfs 6,107 cf scarded=0.89 cfs 6,103 cf Primary=0.00 cfs 0 cf Outflow=0.89 cfs 6,103 cf
Pond IB-2: Infiltration Basin #2	Peak Elev=212.62' Storage=5,132 cf Inflow=7.02 cfs 20,262 cf arded=1.65 cfs 20,243 cf Primary=0.00 cfs 0 cf Outflow=1.65 cfs 20,243 cf
Pond IB-3: Infiltration Basin #3 Di	Peak Elev=213.65' Storage=996 cf Inflow=2.86 cfs 8,242 cf scarded=1.28 cfs 8,237 cf Primary=0.00 cfs 0 cf Outflow=1.28 cfs 8,237 cf
Pond IB-4: Infiltration Basin #4	Peak Elev=212.00' Storage=3,541 cf Inflow=5.37 cfs 15,545 cf arded=1.41 cfs 15,533 cf Primary=0.00 cfs 0 cf Outflow=1.41 cfs 15,533 cf
Total Runoff Area = 2	,264,103 sf Runoff Volume = 62,807 cf Average Runoff Depth = 0.33" 94.00% Pervious = 2,128,182 sf 6.00% Impervious = 135,921 sf

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## Summary for Subcatchment PR-1A: South of stream

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 5.00 hrs, Volume=

0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

A	rea (sf)	CN I	Description		
8	97,676	30 \	Woods, Go	od, HSG A	
	47,438	68 ·	<50% Gras	s cover, Po	or, HSG A
	1,947	98 I	Paved park	ing, HSG A	
9	47,061	32 \	Weighted A	verage	
9	45,114	ę	99.79% Pe	rvious Area	
	1,947	(	0.21% Impe	ervious Area	а
-		01		0	
Tc	Length	Slope		Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
31.5	50	0.0020	0.03		Sheet Flow, A-B
					Woods: Light underbrush n= 0.400 P2= 3.10"
73.4	985	0.0020	0.22		Shallow Concentrated Flow, B-C
					Woodland Kv= 5.0 fps
101 0	1 025	Total			

104.9 1,035 Total

# Summary for Subcatchment PR-1B: North of stream

[73] Warning: Peak may fall outside time span

Runoff = 0.02 cfs @ 20.00 hrs, Volume= 69 cf, Depth> 0.00"

_	А	rea (sf)	CN D	escription			
	6	46,242	30 V	loods, Go	od, HSG A		
		67,899	68 <	50% Grass	s cover, Po	or, HSG A	
_		33,279	30 N	leadow, no	on-grazed,	HSG A	
	7	47,420	33 V	eighted A	verage		
	7	47,420	1	00.00% Pe	ervious Are	а	
	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	5.7	50	0.0200	0.15		Sheet Flow, A-B	
						Grass: Short n= 0.150 P2= 3.10"	
	38.2	810	0.0050	0.35		Shallow Concentrated Flow, B-C	
_						Woodland Kv= 5.0 fps	
	43.9	860	Total				

22-243 Post Development - Rev 2 Type III 24-hr 10 Year Rainfall=4.44" Prepared by Landtech Consultants, Inc. Printed 1/14/2025 HydroCAD® 10.00-26 s/n 00798 © 2020 HydroCAD Software Solutions LLC

## Summary for Subcatchment PR-1C: Jenkins - entrance

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[49] Hint: Tc<2dt may require smaller dt

= 2.12 cfs @ 12.08 hrs, Volume= 6,107 cf, Depth> 2.25" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

Ar	ea (sf)	CN	Description				
	9,869	98	Paved road	s w/curbs &	& sewers, HSG A		
-	19,113	68	<50% Gras	s cover, Po	bor, HSG A		
	3,635	98	Roofs, HSC	βA			
3	32,617	80	Weighted A	verage			
	19,113		58.60% Pervious Area				
	13,504		41.40% Impervious Area				
Tc (min)	Length (feet)	Slop (ft/ft		Capacity (cfs)	Description		
5.0					Direct Entry,		

## Summary for Subcatchment PR-1D: Jenkins - Middle

[49] Hint: Tc<2dt may require smaller dt

Runoff 7.02 cfs @ 12.08 hrs, Volume= 20,262 cf, Depth> 2.33" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

Area (sf)	CN	Description						
34,458	98	Paved road	s w/curbs &	& sewers, HSG A				
60,702	68	<50% Gras	s cover, Po	bor, HSG A				
9,198	98	Roofs, HSC	βA					
104,358	81	Weighted A	Weighted Average					
60,702		58.17% Per	58.17% Pervious Area					
43,656		41.83% Imp	pervious Ar	ea				
Tc Length	Slop	,	Capacity	Description				
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)					
5.0				Direct Entry,				

## Summary for Subcatchment PR-1E: Gratuity Rd - middle near entrance

[49] Hint: Tc<2dt may require smaller dt

2.86 cfs @ 12.08 hrs, Volume= 8,242 cf, Depth> 2.17" Runoff =

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Α	rea (sf)	CN	Description					
	15,364	98	Paved road	s w/curbs &	& sewers, HSG A			
	28,536	68	<50% Gras	s cover, Po	oor, HSG A			
	1,785	98	Roofs, HSC	βA				
	45,685	79	Weighted Average					
	28,536		62.46% Pervious Area					
	17,149		37.54% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	1			
5.0					Direct Entry,			

# Summary for Subcatchment PR-1F: Gratuity Rd - cul-de-sac

[49] Hint: Tc<2dt may require smaller dt

Runoff = 5.37 cfs @ 12.08 hrs, Volume= 15,545 cf, Depth> 2.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

A	rea (sf)	CN	Description					
	27,603	98	Paved road	s w/curbs &	& sewers, HSG A			
	42,235	68	<50% Gras	s cover, Po	oor, HSG A			
	7,414	98	Roofs, HSC	βA				
	77,252	82	Weighted Average					
	42,235		54.67% Pervious Area					
	35,017		45.33% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	1			
5.0			· · ·		Direct Entry,			

# Summary for Subcatchment PR-1G: (new Subcat)

Runoff = 0.05 cfs @ 14.81 hrs, Volume= 1,016 cf, Depth> 0.09"

 Area (sf)	CN	Description				
33,650	68	<50% Grass cover, Poor, HSG A				
 97,543	30	Woods, Good, HSG A				
131,193	40	Weighted Average				
131,193		100.00% Pervious Area				

			<b>ient - Re</b> onsultant		Type III 24-hr 10 Year Rainfall=4.44" Printed 1/14/2025		
					D Software Solutions LLC Page 38		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
16.5	50	0.0100	0.05	(	Sheet Flow,		
8.3	250	0.0100	0.50		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps		
24.8	300	Total					
		Su	mmary f	or Subcat	tchment PR-1H: (new Subcat)		
Runoff	=	0.05 cfs	s@ 13.9	3 hrs, Volu	ume= 1,040 cf, Depth> 0.12"		
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr  10 Year Rainfall=4.44"						
A	rea (sf)	CN D	escription				
	29,892				bor, HSG A		
	<u>77,876</u> 07,768		Veighted A	od, HSG A verage	·		
	07,768			ervious Are	a		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
16.5	<u>(1001)</u> 50	0.0100	0.05	(013)	Sheet Flow,		
8.3	250	0.0100	0.50		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps		
24.8	300	Total					
			Sum	mary for	Subcatchment PR-1I:		
[49] Hint	: Tc<2dt	may requ	ire smaller	dt			
Runoff	=	0.17 cf	s@ 12.1	1 hrs, Volu	ume= 666 cf, Depth> 0.58"		
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr  10 Year Rainfall=4.44"						
A	rea (sf)		escription				
	8,676 5,002			s cover, Pc od, HSG A	bor, HSG A		
	<u>3,002</u> 13,678		Veighted A	,			
	13,678			ervious Are	a		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

## Summary for Subcatchment PR-1K:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.02 cfs @ 12.42 hrs, Volume= 242 cf, Depth> 0.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

A	rea (sf)	CN	Description				
	5,678	68	<50% Gras	s cover, Po	bor, HSG A		
	11,311	30	Meadow, no	on-grazed, l	HSG A		
	16,989	43	Weighted Average				
	16,989		100.00% Pervious Area				
Тс	Length	Slope		Capacity	Description		
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)			
5.0					Direct Entry,		

#### Summary for Subcatchment PR-2: Gratuity Rd - entrance

Runoff	=	0.74 cfs @	12.15 hrs,	Volume=	2,463 cf, Depth> 1.63"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

_	A	rea (sf)	CN E	Description						
		2,680	98 F	Paved roads w/curbs & sewers, HSG A						
_		15,434	68 <	<50% Gras	s cover, Po	or, HSG A				
		18,114	72 V	Veighted A	verage					
		15,434	8	35.20% Per	rvious Area					
		2,680	1	4.80% Imp	pervious Are	ea				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	7.5	50	0.0100	0.11		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.10"				
	2.3	98	0.0100	0.70		Shallow Concentrated Flow,				
_						Short Grass Pasture Kv= 7.0 fps				
	0 0	110	Total							

9.8 148 Total

## Summary for Subcatchment Unit 1: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff 0.18 cfs @ 12.07 hrs, Volume= 581 cf, Depth> 3.91" =

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Area (sf)	CN	Description					
1,785	98	98 Roofs, HSG A					
1,785		100.00% Impervious Area					
Tc Length (min) (feet)	Slope (ft/ft		Capacity (cfs)	Description			
5.0				Direct Entry,			

## Summary for Subcatchment Unit 10: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 605 cf, Depth> 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

Ar	ea (sf)	CN	Description					
	1,857	98	Roofs, HSG A					
	1,857		100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description			
5.0					Direct Entry,			

# Summary for Subcatchment Unit 11: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 603 cf, Depth> 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

A	rea (sf)	CN I	Description					
	1,850	98 I	98 Roofs, HSG A					
	1,850		100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

# Summary for Subcatchment Unit 12: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 605 cf, Depth> 3.91"

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A	rea (sf)	CN	Description					
	1,857	98	98 Roofs, HSG A					
	1,857		100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)				
5.0					Direct Entry,			

## Summary for Subcatchment Unit 2: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 603 cf, Depth> 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

A	rea (sf)	CN	CN Description					
	1,850	98	8 Roofs, HSG A					
	1,850		100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
5.0					Direct Entry,			

# Summary for Subcatchment Unit 3: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 605 cf, Depth> 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

Area (sf	) CN	Description					
1,85	7 98	98 Roofs, HSG A					
1,85	7	100.00% Impervious Area					
Tc Leng (min) (fee		,	Capacity (cfs)	Description			
5.0				Direct Entry,			

# Summary for Subcatchment Unit 4: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.18 cfs @ 12.07 hrs, Volume=

581 cf, Depth> 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

A	rea (sf)	CN	Description				
	1,785	98	Roofs, HSC	βA			
	1,785	5 100.00% Impervious Area					
Tc (min)	Length (feet)	Slop (ft/fl		Capacity (cfs)	Description		
5.0	()	(101)	., (	(0.0)	Direct Entry,		
		_	_	_			

## Summary for Subcatchment Unit 5: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 603 cf, Depth> 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

A	rea (sf)	CN I	Description				
	1,850	98	Roofs, HSC	θA			
	1,850	100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

# Summary for Subcatchment Unit 6: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 605 cf, Depth> 3.91"

A	rea (sf)	CN I	Description			
	1,857	98 I	Roofs, HSC	βA		
	1,857	100.00% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)		
5.0					Direct Entry,	

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## Summary for Subcatchment Unit 7: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.18 cfs @ 12.07 hrs, Volume= 581 cf, Depth> 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

Α	rea (sf)	CN	Description				
	1,785	98	Roofs, HSG A				
	1,785		100.00% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft	e Velocity ) (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

## Summary for Subcatchment Unit 8: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.18 cfs @ 12.07 hrs, Volume= 581 cf, Depth> 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 Year Rainfall=4.44"

A	rea (sf)	CN	Description						
	1,785	98	98 Roofs, HSG A						
	1,785		100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

## Summary for Subcatchment Unit 9: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 603 cf, Depth> 3.91"

 Area (sf)	CN	Description
1,850	98	Roofs, HSG A
1,850		100.00% Impervious Area

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Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
5.0					Direct Entry,

## Summary for Reach DP-1: Stream

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =	1,694,481 sf,	0.11% Impervious,	Inflow Depth > 0.00"	for 10 Year event
Inflow =	0.02 cfs @ 2	20.00 hrs, Volume=	69 cf	
Outflow =	0.02 cfs @ 2	20.00 hrs, Volume=	69 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

# Summary for Reach DP-2: Gratuity Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	18,114 sf, 14.80% Impervious, Inflow Depth > 1.63" for 10 Year event	t
Inflow	=	0.74 cfs @ 12.15 hrs, Volume= 2,463 cf	
Outflow	=	0.74 cfs @ 12.15 hrs, Volume= 2,463 cf, Atten= 0%, Lag= 0.0 m	in

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

## Summary for Pond DE-1: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage

Inflow Area =	1,785 sf,100.00% Impervious,	Inflow Depth > 3.91" for 10 Year event
Inflow =	0.18 cfs @ 12.07 hrs, Volume=	581 cf
Outflow =	0.07 cfs @ 11.95 hrs, Volume=	581 cf, Atten= 58%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.95 hrs, Volume=	581 cf
Primary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.39' @ 12.26 hrs Surf.Area= 0.009 ac Storage= 0.001 af

Plug-Flow detention time= 3.6 min calculated for 579 cf (100% of inflow) Center-of-Mass det. time= 3.5 min (738.6 - 735.1)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	0.007 af	<b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids
Device	Routing	Invert Ou	utlet Devices
#1	Discarded	0.00' <b>8.</b> 2	270 in/hr Exfiltration over Surface area
#2	Primary	He 2.8	A.O' long x 2.0' breadth Broad-Crested Rectangular Weir           ead (feet)         0.20         0.40         0.60         0.80         1.00         1.20         1.40         1.60         1.80         2.00           50         3.00         3.50         3.50         2.61         2.60         2.66         2.70         2.77         2.89         2.88

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2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.07 cfs @ 11.95 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## Summary for Pond DE-10: Drip Edge

[82] Warning: Early inflow requires earlier time span[92] Warning: Device #2 is above defined storage

Inflow Area =	1,857 sf,100.00% Impervious,	Inflow Depth > 3.91" for 10 Year event
Inflow =	0.19 cfs @ 12.07 hrs, Volume=	605 cf
Outflow =	0.07 cfs @ 11.95 hrs, Volume=	605 cf, Atten= 60%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.95 hrs, Volume=	605 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.43' @ 12.27 hrs Surf.Area= 0.009 ac Storage= 0.002 af

Plug-Flow detention time= 4.0 min calculated for 603 cf (100% of inflow) Center-of-Mass det. time= 3.8 min (739.0 - 735.1)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	0.007 af	<b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids
Device	Routing	Invert Ou	utlet Devices
#1	Discarded	0.00' <b>8.2</b>	270 in/hr Exfiltration over Surface area
#2	Primary		.0' long x 2.0' breadth Broad-Crested Rectangular Weir
		He	ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
		2.5	50 3.00 3.50
		Co	pef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
		2.8	85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.07 cfs @ 11.95 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## Summary for Pond DE-11: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

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Inflow Area =       1,850 sf,100.00% Impervious, Inflow Depth > 3.91" for 10 Year event         Inflow =       0.19 cfs @ 12.07 hrs, Volume=       603 cf         Outflow =       0.07 cfs @ 11.90 hrs, Volume=       602 cf, Atten= 63%, Lag= 0.0 min         Discarded =       0.07 cfs @ 11.90 hrs, Volume=       602 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf			
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.52' @ 12.30 hrs Surf.Area= 0.008 ac Storage= 0.002 af			
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 4.7 min(739.8 - 735.1)			
VolumeInvertAvail.StorageStorage Description#10.00'0.007 af <b>3.00'W x 120.00'L x 2.00'H Prismatoid</b> 0.017 af Overall x 40.0% Voids			
Device         Routing         Invert         Outlet Devices           #1         Discarded         0.00'         8.270 in/hr Exfiltration over Surface area           #2         Primary         2.00'         72.0' long x 2.0' breadth Broad-Crested Rectangular Weir           Head (feet)         0.20         0.40         0.60         0.80         1.00         1.20         1.40         1.60         1.80         2.00'           2.50         3.00         3.50         Coef. (English)         2.54         2.61         2.61         2.66         2.70         2.77         2.89         2.88           2.85         3.07         3.20         3.32         3.32         3.32         3.32			
Discarded OutFlow Max=0.07 cfs @ 11.90 hrs HW=0.02' (Free Discharge) ☐ 1=Exfiltration (Exfiltration Controls 0.07 cfs)			
Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) <sup>●</sup> 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)			
Summary for Pond DE-12: Drip Edge			
[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage			
Inflow Area =       1,857 sf,100.00% Impervious, Inflow Depth > 3.91" for 10 Year event         Inflow =       0.19 cfs @ 12.07 hrs, Volume=       605 cf         Outflow =       0.07 cfs @ 11.95 hrs, Volume=       605 cf, Atten= 60%, Lag= 0.0 min         Discarded =       0.07 cfs @ 11.95 hrs, Volume=       605 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf			
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.43' @ 12.27 hrs Surf.Area= 0.009 ac Storage= 0.002 af			
Plug-Flow detention time= 4.0 min calculated for 603 cf (100% of inflow) Center-of-Mass det. time= 3.8 min(739.0 - 735.1)			
VolumeInvertAvail.StorageStorage Description#10.00'0.007 af <b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids			

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Device	Routing	Invert	Outlet Devices	
#1 #2	Discarded Primary	0.00' 2.00'	8.270 in/hr Exfiltration over Surface area 72.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32	
	ed OutFlow Max filtration (Exfiltra		s @ 11.95 hrs HW=0.03' (Free Discharge) htrols 0.07 cfs)	
			@ 5.00 hrs HW=0.00' (Free Discharge) r <b>Weir</b> ( Controls 0.00 cfs)	
		S	ummary for Pond DE-2: Drip Edge	
	rning: Early inflow rning: Device #2 i		s earlier time span defined storage	
Inflow Outflow Discarde Primary Routing	Outflow= $0.06 \text{ cfs}$ 11.85 hrs, Volume= $602 \text{ cf}$ , Atten= 66%, Lag= 0.0 minDiscarded= $0.06 \text{ cfs}$ 11.85 hrs, Volume= $602 \text{ cf}$ Primary= $0.00 \text{ cfs}$ $5.00 \text{ hrs}$ , Volume= $0 \text{ cf}$ Routing by Stor-Ind method, Time Span= $5.00-20.00 \text{ hrs}$ , dt= $0.05 \text{ hrs}$ Peak Elev= $0.64'$ $0 \text{ 12.33 hrs}$ Surf.Area= $0.008 \text{ ac}$ Storage= $0.002 \text{ af}$			
			n ( 741.0 - 735.1 )	
Volume			age Storage Description	
#1	0.00'	0.006	6 af <b>3.00'W x 110.00'L x 2.00'H Prismatoid</b> 0.015 af Overall x 40.0% Voids	
Device	Routing	Invert	Outlet Devices	
#1 #2	Discarded Primary	0.00' 2.00'		
	Discarded OutFlow Max=0.06 cfs @ 11.85 hrs HW=0.02' (Free Discharge) ↑ 1=Exfiltration (Exfiltration Controls 0.06 cfs)			

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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# Summary for Pond DE-3: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage

Inflow Area =	1,857 sf,100.00% Impervious,	Inflow Depth > 3.91" for 10 Year event
Inflow =	0.19 cfs @ 12.07 hrs, Volume=	605 cf
Outflow =	0.07 cfs @ 11.90 hrs, Volume=	605 cf, Atten= 65%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.90 hrs, Volume=	605 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.58' @ 12.32 hrs Surf.Area= 0.008 ac Storage= 0.002 af

Plug-Flow detention time= 5.5 min calculated for 603 cf (100% of inflow) Center-of-Mass det. time= 5.3 min (740.4 - 735.1)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	0.006 af	<b>3.00'W x 115.00'L x 2.00'H Prismatoid</b> 0.016 af Overall x 40.0% Voids
Device	Routing	Invert O	utlet Devices
#1	Discarded	0.00' <b>8.</b>	270 in/hr Exfiltration over Surface area
#2	Primary	H 2. C	2.0' long x 2.0' breadth Broad-Crested Rectangular Weir         ead (feet)       0.20       0.40       0.60       0.80       1.00       1.20       1.40       1.60       1.80       2.00         50       3.00       3.50       0       0       0.61       2.60       2.66       2.70       2.77       2.89       2.88         85       3.07       3.20       3.32

**Discarded OutFlow** Max=0.07 cfs @ 11.90 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond DE-4: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area =	1,785 sf,100.00% Impervious,	Inflow Depth > 3.91" for 10 Year event
Inflow =	0.18 cfs @ 12.07 hrs, Volume=	581 cf
Outflow =	0.06 cfs @ 11.85 hrs, Volume=	581 cf, Atten= 68%, Lag= 0.0 min
Discarded =	0.06 cfs @ 11.85 hrs, Volume=	581 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.73' @ 12.36 hrs Surf.Area= 0.007 ac Storage= 0.002 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)

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Center-of-Mass det. time= 6.9 min (742.0 - 735.1)

Volume	Invert	Avail.Storage	Storage Description		
#1	0.00'	0.006 af			
			0.014 af Overall x 40.0% Voids		
Device	Routing	Invert Ou	tlet Devices		
#1 #2	Discarded Primary	2.00' <b>72.</b> He 2.5 Co	<b>72.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88		
Ê—1=Ex	filtration (Exf	lax=0.06 cfs @ iltration Controls	35 3.07 3.20 3.32 11.85 hrs HW=0.03' (Free Discharge) s 0.06 cfs) 00 hrs HW=0.00' (Free Discharge)		
			ir (Controls 0.00 cfs)		
		Sumr	nary for Pond DE-5: Drip Edge		
		low requires ear 2 is above defir			
Inflow A Inflow Outflow Discarde Primary	= 0.1 = 0.0 ed = 0.0	9 cfs @ 12.07	hrs, Volume= 602 cf, Atten= 63%, Lag= 0.0 min hrs, Volume= 602 cf		
			n= 5.00-20.00 hrs, dt= 0.05 hrs rea= 0.008 ac Storage= 0.002 af		
		ne= (not calcula ne= 4.7 min ( 73	ted: outflow precedes inflow) 39.8 - 735.1)		
Volume	Invert	Avail.Storage	Storage Description		
#1	0.00'	0.007 af	<b>3.00'W x 120.00'L x 2.00'H Prismatoid</b> 0.017 af Overall x 40.0% Voids		
Device	Routing	Invert Ou	tlet Devices		
#1 #2	Discarded Primary	2.00' <b>72.</b> He 2.5 Co	70 in/hr Exfiltration over Surface area 0' long x 2.0' breadth Broad-Crested Rectangular Weir ad (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 i0 3.00 3.50 ef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 i5 3.07 3.20 3.32		

**Discarded OutFlow** Max=0.07 cfs @ 11.90 hrs HW=0.02' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond DE-6: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage

Inflow Area =	1,857 sf,100.00% Impervious,	Inflow Depth > 3.91" for 10 Year event
Inflow =	0.19 cfs @ 12.07 hrs, Volume=	605 cf
Outflow =	0.07 cfs @ 11.95 hrs, Volume=	605 cf, Atten= 60%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.95 hrs, Volume=	605 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.43' @ 12.27 hrs Surf.Area= 0.009 ac Storage= 0.002 af

Plug-Flow detention time= 4.0 min calculated for 603 cf (100% of inflow) Center-of-Mass det. time= 3.8 min (739.0 - 735.1)

Volume	Invert	Avail.Storag	e Storage Description
#1	0.00'	0.007 a	af <b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids
Device	Routing	Invert (	Outlet Devices
<u></u> #1	Discarded		8.270 in/hr Exfiltration over Surface area
#2	Primary		72.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.20 3.32
		4	2.05 5.07 5.20 5.52

**Discarded OutFlow** Max=0.07 cfs @ 11.95 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond DE-7: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

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Inflow Area =       1,785 sf,100.00% Impervious, Inflow Depth > 3.91" for 10 Year event         Inflow =       0.18 cfs @ 12.07 hrs, Volume=       581 cf         Outflow =       0.06 cfs @ 11.90 hrs, Volume=       581 cf, Atten= 65%, Lag= 0.0 min         Discarded =       0.06 cfs @ 11.90 hrs, Volume=       581 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf				
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.59' @ 12.32 hrs Surf.Area= 0.008 ac Storage= 0.002 af				
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 5.3 min ( 740.5 - 735.1 )				
Volume Invert Avail.Storage Storage Description				
#1 0.00' 0.006 af <b>3.00'W x 110.00'L x 2.00'H Prismatoid</b> 0.015 af Overall x 40.0% Voids				
Device Routing Invert Outlet Devices				
#1 Discarded 0.00' 8.270 in/hr Exfiltration over Surface area				
#2       Primary       2.00'       72.0' long x 2.0' breadth Broad-Crested Rectangular Weir				
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50				
Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88				
2.85 3.07 3.20 3.32				
Discarded OutFlow Max=0.06 cfs @ 11.90 hrs HW=0.03' (Free Discharge) ←1=Exfiltration (Exfiltration Controls 0.06 cfs)				
Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) <sup>●</sup> 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)				
Summary for Pond DE-8: Drip Edge				
[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage				
Inflow Area = 1,785 sf,100.00% Impervious, Inflow Depth > 3.91" for 10 Year event				
Inflow = 0.18 cfs @ 12.07 hrs, Volume= 581 cf				
Outflow         =         0.07 cfs @         11.95 hrs, Volume=         581 cf, Atten= 58%, Lag= 0.0 min           Discarded         =         0.07 cfs @         11.95 hrs, Volume=         581 cf				
Primary = $0.00 \text{ cfs} @ 5.00 \text{ hrs}, \text{ Volume} = 0 \text{ cf}$				
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.39' @ 12.26 hrs Surf.Area= 0.009 ac Storage= 0.001 af				
Plug-Flow detention time= 3.6 min calculated for 579 cf (100% of inflow) Center-of-Mass det. time= 3.5 min ( 738.6 - 735.1 )				
Volume Invert Avail.Storage Storage Description				
#1 0.00' 0.007 af <b>3.00'W x 130.00'L x 2.00'H Prismatoid</b>				
0.018 af Overall x 40.0% Voids				

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Device Routing Invert Outle	t Devices			
#1 Discarded 0.00' <b>8.270</b> #2 Primary 2.00' <b>72.0'</b> Head 2.50 Coef.	in/hr Exfiltration over Surface area long x 2.0' breadth Broad-Crested Rectangular Weir (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 3.00 3.50 (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 3.07 3.20 3.32			
Discarded OutFlow Max=0.07 cfs @ 11 ☐ 1=Exfiltration (Exfiltration Controls 0				
Primary OutFlow Max=0.00 cfs @ 5.00 <sup>▲</sup> 2=Broad-Crested Rectangular Weir				
Summa	ry for Pond DE-9: Drip Edge			
[82] Warning: Early inflow requires earlie [92] Warning: Device #2 is above defined [85] Warning: Oscillations may require sr	l storage			
Inflow Area =       1,850 sf,100.00% Impervious, Inflow Depth > 3.91" for 10 Year event         Inflow =       0.19 cfs @ 12.07 hrs, Volume=       603 cf         Outflow =       0.06 cfs @ 11.85 hrs, Volume=       602 cf, Atten= 68%, Lag= 0.0 min         Discarded =       0.06 cfs @ 11.85 hrs, Volume=       602 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf				
Routing by Stor-Ind method, Time Span= Peak Elev= 0.76' @ 12.36 hrs Surf.Area				
Plug-Flow detention time= (not calculated Center-of-Mass det. time= 7.1 min ( 742.				
Volume Invert Avail.Storage S	torage Description			
#1 0.00' 0.006 af <b>3</b>	.00'W x 102.00'L x 2.00'H Prismatoid .014 af Overall x 40.0% Voids			
Device Routing Invert Outle	t Devices			
#2 Primary 2.00' <b>72.0'</b> Head 2.50 Coef.	in/hr Exfiltration over Surface area long x 2.0' breadth Broad-Crested Rectangular Weir (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 3.00 3.50 (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 3.07 3.20 3.32			
<b>Discarded OutFlow</b> Max=0.06 cfs @ 11				
Primary OutFlow Max=0.00 cfs @ 5.00				

**1**–2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond GD-1: Ground Depression

Inflow Area = Inflow = Outflow = Discarded =	0.05 cfs @ 14 0.05 cfs @ 14	0.00% Imperviou 4.81 hrs, Volume 4.81 hrs, Volume 4.81 hrs, Volume	e= 1,015 cf, Atten= 0%, Lag= 0.3 min			
	Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 211.60' @ 14.81 hrs Surf.Area= 1,533 sf Storage= 1 cf					
Plug-Flow detentic Center-of-Mass de	et. time= 0.2 mir	(962.7 - 962.4)				
Volume Inve	ert Avail.Sto	rage Storage De	escription			
#1 211.6	0' 2,90	03 cf Custom St	tage Data (Prismatic) Listed below (Recalc)			
Elevation	Surf.Area	Inc.Store	Cum.Store			
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)			
211.60	1,514	0	0			
212.00	13,000	2,903	2,903			
Device Routing	Invert	Outlet Devices				
#1 Discarde	d 211.60'	8.270 in/hr Exfil	Itration over Surface area			

**Discarded OutFlow** Max=0.29 cfs @ 14.81 hrs HW=211.60' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.29 cfs)

## Summary for Pond GD-2: Ground Depression

Inflow Area =	107,768 sf, 0.00% Impervious	, Inflow Depth > 0.12" for 10 Year event
Inflow =	0.05 cfs @ 13.93 hrs, Volume=	1,040 cf
Outflow =	0.05 cfs @ 13.95 hrs, Volume=	1,039 cf, Atten= 0%, Lag= 0.7 min
Discarded =	0.05 cfs @ 13.95 hrs, Volume=	1,039 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 210.60' @ 13.95 hrs Surf.Area= 575 sf Storage= 2 cf

Plug-Flow detention time= 0.7 min calculated for 1,039 cf (100% of inflow) Center-of-Mass det. time= 0.5 min (950.0 - 949.5)

Volume	Invert	Avail.Sto	rage Storag	ge Description	
#1	210.60'	3,54	47 cf Custo	om Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
210.6	60	550	0	0	
211.0	00	2,998	710	710	
211.5	50	8,352	2,838	3,547	
Device	Routing	Invert	Outlet Devi	ces	
#1	Discarded	210.60'	8.270 in/hr	Exfiltration over	Surface area

**Discarded OutFlow** Max=0.11 cfs @ 13.95 hrs HW=210.60' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.11 cfs)

## Summary for Pond GD-3: Ground Depression

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Inflow Area =	13,678 sf, 0.00% Impervious,	Inflow Depth > 0.58" for 10 Year event
Inflow =	0.17 cfs @ 12.11 hrs, Volume=	666 cf
Outflow =	0.09 cfs @ 12.41 hrs, Volume=	666 cf, Atten= 47%, Lag= 18.1 min
Discarded =	0.09 cfs @ 12.41 hrs, Volume=	666 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.75' @ 12.41 hrs Surf.Area= 469 sf Storage= 61 cf

Plug-Flow detention time= 3.6 min calculated for 666 cf (100% of inflow) Center-of-Mass det. time= 3.4 min (855.9 - 852.5)

Volume	Invert	Avail.Sto	rage Stora	age Description
#1	213.60'	91	19 cf Cus	tom Stage Data (Prismatic) Listed below (Recalc)
Elevatior (feet		ırf.Area (sq-ft)	Inc.Store (cubic-feet	
213.60	)	355	(	0 0
214.00	)	660	203	3 203
214.50	)	2,205	716	6 919
Device	Routing	Invert	Outlet De	vices
#1	Discarded	213.60'	8.270 in/h	nr Exfiltration over Surface area

**Discarded OutFlow** Max=0.09 cfs @ 12.41 hrs HW=213.75' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.09 cfs)

# Summary for Pond GD-4: Ground Depression

Inflow Area =	16,989 sf, 0.00% Impervie	ous, Inflow Depth > 0.17"	for 10 Year event
Inflow =	0.02 cfs @ 12.42 hrs, Volun	ne= 242 cf	
Outflow =	0.02 cfs @ 12.42 hrs, Volun	ne= 242 cf, Atte	en= 0%, Lag= 0.3 min
Discarded =	0.02 cfs @ 12.42 hrs, Volun	ne= 242 cf	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.60' @ 12.42 hrs Surf.Area= 551 sf Storage= 0 cf

Plug-Flow detention time= 0.3 min calculated for 242 cf (100% of inflow) Center-of-Mass det. time= 0.2 min (916.3 - 916.1)

Volume	Invert	Avail.Storage	Storage Description
#1	213.60'	363 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Type III 24-hr 10 Year Rainfall=4.44" Printed 1/14/2025 SLLC Page 55

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Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
213.0 214.0		550 1,263	0 363	0 363		
Device	Routing	Invert	Outlet Devices			
	<b>D</b> · · ·				~ ~	

#1 Discarded 213.60' 8.270 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.11 cfs @ 12.42 hrs HW=213.60' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.11 cfs)

## Summary for Pond IB-1: Infiltration Basin #1

Inflow Area =	32,617 sf, 41.40% Impervious,	Inflow Depth > 2.25" for 10 Year event
Inflow =	2.12 cfs @ 12.08 hrs, Volume=	6,107 cf
Outflow =	0.89 cfs @ 12.30 hrs, Volume=	6,103 cf, Atten= 58%, Lag= 13.5 min
Discarded =	0.89 cfs @ 12.30 hrs, Volume=	6,103 cf
Primary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 211.18' @ 12.30 hrs Surf.Area= 4,648 sf Storage= 802 cf

Plug-Flow detention time= 5.3 min calculated for 6,103 cf (100% of inflow) Center-of-Mass det. time= 5.1 min (795.5 - 790.5)

Volume	Invert	t Avail.Stor	rage Storage	Description	
#1	211.00'	' 11,23	B6 cf Custom	Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
211.0	00	4,480	0	0	
212.0	00	5,436	4,958	4,958	
213.0	00	7,119	6,278	11,236	
Device	Routing	Invert	Outlet Devices	5	
#1	Primary	212.00'	Head (feet) 0	.20 0.40 0.60	Dad-Crested Rectangular Weir           0.80         1.00         1.20         1.40         1.60           70         2.64         2.64         2.63         2.64         2.63
#2	Discarded	211.00'		filtration over	
213.0 <u>Device</u> #1 #2	0 <u>Routing</u> Primary Discarded	7,119 <u>Invert</u> 212.00' 211.00'	6,278 Outlet Devices 5.0' long x 15 Head (feet) 0 Coef. (English 8.270 in/hr Ex	11,236 5.0' breadth Bro .20 0.40 0.60 ) 2.68 2.70 2. filtration over \$	0.80 1.00 1.20 1.40 1.60 .70 2.64 2.63 2.64 2.64 2.63

**Discarded OutFlow** Max=0.89 cfs @ 12.30 hrs HW=211.18' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.89 cfs)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=211.00' (Free Discharge) **1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

# Summary for Pond IB-2: Infiltration Basin #2

Inflow Area =	104,358 sf, 41.83% Impervious,	Inflow Depth > 2.33" for 10 Year event
Inflow =	7.02 cfs @ 12.08 hrs, Volume=	20,262 cf
Outflow =	1.65 cfs @ 12.49 hrs, Volume=	20,243 cf, Atten= 77%, Lag= 24.6 min
Discarded =	1.65 cfs @ 12.49 hrs, Volume=	20,243 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 212.62' @ 12.49 hrs Surf.Area= 8,605 sf Storage= 5,132 cf

Plug-Flow detention time= 20.0 min calculated for 20,243 cf (100% of inflow) Center-of-Mass det. time= 19.6 min (807.8 - 788.2)

Volume	Inver	t Avail.Stor	age Storage	e Description				
#1	212.00	' 28,93	9 cf Custon	n Stage Data (Pri	<b>smatic)</b> Listed below (Recalc)			
Elevatio	n S	urf.Area	Inc.Store	Cum.Store				
(feet		(sq-ft)	(cubic-feet)	(cubic-feet)				
212.0	0	7,901	0	0				
213.0	0	9,033	8,467	8,467				
214.0	0	10,222	9,628	18,095				
215.0	0	11,467	10,845	28,939				
Device	Routing	Invert	Outlet Device	es				
#1	Discarded	212.00'	8.270 in/hr E	xfiltration over S	Surface area			
#2	Primary	214.00'	5.0' long x 1	5.0' breadth Bro	ad-Crested Rectangular Weir			
			Head (feet)	0.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60			
			Coef. (Englis	sh) 2.68 2.70 2.	70 2.64 2.63 2.64 2.64 2.63			
	<b>Discarded OutFlow</b> Max=1.65 cfs @ 12.49 hrs HW=212.62' (Free Discharge)							

-1=Exfiltration (Exfiltration Controls 1.65 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=212.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond IB-3: Infiltration Basin #3

Inflow Area =	45,685 sf, 37.54% Impervious,	Inflow Depth > 2.17" for 10 Year event
Inflow =	2.86 cfs @ 12.08 hrs, Volume=	8,242 cf
Outflow =	1.28 cfs @ 12.28 hrs, Volume=	8,237 cf, Atten= 55%, Lag= 12.1 min
Discarded =	1.28 cfs @ 12.28 hrs, Volume=	8,237 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.65' @ 12.28 hrs Surf.Area= 6,673 sf Storage= 996 cf

Plug-Flow detention time= 4.7 min calculated for 8,210 cf (100% of inflow) Center-of-Mass det. time= 4.4 min (797.1 - 792.7)

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Volume	Invert Avail.	Storage Storag	ge Description	
#1 2		<u> </u>		ismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
213.50 214.00	6,490 7,094	0 3,396	0 3,396	
214.50	7,710	3,701	7,097	
215.00 215.50	8,343 9,940	4,013 4,571	11,110 15,681	
Device Rout	ng Inve	ert Outlet Devi	ces	
#1 Disca #2 Prima	arded 213.5 ary 214.5	50' <b>5.0' long x</b> Head (feet) 2.50 3.00 Coef. (Engl	0.20 0.40 0.60 3.50 4.00 4.50	ad-Crested Rectangular Weir 0.80 1.00 1.20 1.40 1.60 1.80 2.00 68 2.67 2.65 2.64 2.64 2.68 2.68

**Discarded OutFlow** Max=1.28 cfs @ 12.28 hrs HW=213.65' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.28 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=213.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond IB-4: Infiltration Basin #4

Inflow Area =	77,252 sf, 45.33% Impervious,	Inflow Depth > 2.41" for 10 Year event
Inflow =	5.37 cfs @ 12.08 hrs, Volume=	15,545 cf
Outflow =	1.41 cfs @ 12.46 hrs, Volume=	15,533 cf, Atten= 74%, Lag= 22.8 min
Discarded =	1.41 cfs @ 12.46 hrs, Volume=	15,533 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 212.00' @ 12.46 hrs Surf.Area= 7,368 sf Storage= 3,541 cf

Plug-Flow detention time= 15.3 min calculated for 15,533 cf (100% of inflow) Center-of-Mass det. time= 15.0 min ( 800.8 - 785.8 )

Volume	Invert	Avai	il.Storage	Storage	e Description	
#1	211.50'	:	21,638 cf	Custor	n Stage Data (Pri	<b>smatic)</b> Listed below (Recalc)
Elevation (feet)		.Area sq-ft)		.Store c-feet)	Cum.Store (cubic-feet)	
211.50		5,864	•	0	0	
212.00	7	7,370		3,559	3,559	
213.00	ç	9,144		8,257	11,816	
214.00	10	0,501		9,823	21,638	

 Type III 24-hr
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Device Routing Invert Outlet Devices	
#1 Discarded 211.50' 8.270 in/hr Exfiltration over Surface are	a
#2 Primary 213.00' 5.0' long x 15.0' breadth Broad-Crested	l Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00	1.20 1.40 1.60
Coef. (English) 2.68 2.70 2.70 2.64 2.	63 2.64 2.64 2.63

**Discarded OutFlow** Max=1.41 cfs @ 12.46 hrs HW=212.00' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.41 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=211.50' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

<b>22-243 Post Development - Rev 2</b> Prepared by Landtech Consultants, Inc. HydroCAD® 10.00-26 s/n 00798 © 2020 Hydroc	<i>Type III 24-hr 25 Year Rainfall=5.55"</i> Printed 1/14/2025 CAD Software Solutions LLC Page 59
Runoff by SCS TR-2	20.00 hrs, dt=0.05 hrs, 301 points 20 method, UH=SCS, Weighted-CN ns method - Pond routing by Stor-Ind method
	Runoff Area=947,061 sf 0.21% Impervious Runoff Depth>0.04" ope=0.0020 '/' Tc=104.9 min CN=32 Runoff=0.19 cfs 3,102 cf
	Runoff Area=747,420 sf 0.00% Impervious Runoff Depth>0.07" w Length=860' Tc=43.9 min CN=33 Runoff=0.23 cfs 4,182 cf
Subcatchment PR-1C: Jenkins - entrance	Runoff Area=32,617 sf 41.40% Impervious Runoff Depth>3.16" Tc=5.0 min CN=80 Runoff=2.96 cfs 8,597 cf
Subcatchment PR-1D: Jenkins - Middle R	unoff Area=104,358 sf 41.83% Impervious Runoff Depth>3.26" Tc=5.0 min CN=81 Runoff=9.72 cfs 28,339 cf
Subcatchment PR-1E: Gratuity Rd - middle	Runoff Area=45,685 sf 37.54% Impervious Runoff Depth>3.07" Tc=5.0 min CN=79 Runoff=4.03 cfs 11,681 cf
Subcatchment PR-1F: Gratuity Rd -	Runoff Area=77,252 sf 45.33% Impervious Runoff Depth>3.36" Tc=5.0 min CN=82 Runoff=7.43 cfs 21,602 cf
	Runoff Area=131,193 sf 0.00% Impervious Runoff Depth>0.30" Slope=0.0100 '/' Tc=24.8 min CN=40 Runoff=0.31 cfs 3,302 cf
	Runoff Area=107,768 sf 0.00% Impervious Runoff Depth>0.34" Glope=0.0100 '/' Tc=24.8 min CN=41 Runoff=0.32 cfs 3,097 cf
Subcatchment PR-1I:	Runoff Area=13,678 sf 0.00% Impervious Runoff Depth>1.07" Tc=5.0 min CN=54 Runoff=0.37 cfs 1,221 cf
Subcatchment PR-1K:	Runoff Area=16,989 sf 0.00% Impervious Runoff Depth>0.44" Tc=5.0 min CN=43 Runoff=0.09 cfs 628 cf
	Runoff Area=18,114 sf 14.80% Impervious Runoff Depth>2.43" Slope=0.0100 '/' Tc=9.8 min CN=72 Runoff=1.11 cfs 3,673 cf
Subcatchment Unit 1: Unit A - Birch	Runoff Area=1,785 sf 100.00% Impervious Runoff Depth>4.92" Tc=5.0 min CN=98 Runoff=0.22 cfs 732 cf
Subcatchment Unit 10: Unit C - Hickory	Runoff Area=1,857 sf 100.00% Impervious Runoff Depth>4.92" Tc=5.0 min CN=98 Runoff=0.23 cfs 761 cf
Subcatchment Unit 11: Unit B - Hemlock	Runoff Area=1,850 sf 100.00% Impervious Runoff Depth>4.92" Tc=5.0 min CN=98 Runoff=0.23 cfs 758 cf
Subcatchment Unit 12: Unit C - Hickory	Runoff Area=1,857 sf 100.00% Impervious Runoff Depth>4.92" Tc=5.0 min CN=98 Runoff=0.23 cfs 761 cf
Subcatchment Unit 2: Unit B - Hemlock	Runoff Area=1,850 sf 100.00% Impervious Runoff Depth>4.92" Tc=5.0 min CN=98 Runoff=0.23 cfs 758 cf

22-243 Post Development - Rev 2Type III 24-hr25 Year Rainfall=5.55"Prepared by Landtech Consultants, Inc.Printed 1/14/2025HydroCAD® 10.00-26 s/n 00798 © 2020 HydroCAD Software Solutions LLCPage 60						
Subcatchment Unit 3: Unit C - Hi	i <b>ckory</b> Ri	unoff Area=1,8			us Runoff Dep Runoff=0.23 cf	
Subcatchment Unit 4: Unit A - Bi	rch R	unoff Area=1,7			us Runoff Dep Runoff=0.22 cf	
Subcatchment Unit 5: Unit B - He	emlock R	unoff Area=1,8			us Runoff Dep Runoff=0.23 cf	
Subcatchment Unit 6: Unit C - Hi	i <b>ckory</b> Ru	unoff Area=1,8			us Runoff Dep Runoff=0.23 cf	
Subcatchment Unit 7: Unit A - Bi	rch R	unoff Area=1,7			us Runoff Dep Runoff=0.22 cf	
Subcatchment Unit 8: Unit A - Bi	rch Ru	unoff Area=1,7			us Runoff Dep Runoff=0.22 cf	
Subcatchment Unit 9: Unit B - He	emlock R	unoff Area=1,8			us Runoff Dep Runoff=0.23 cf	
Reach DP-1: Stream					nflow=0.39 cfs ıtflow=0.39 cfs	
Reach DP-2: Gratuity Road					nflow=1.11 cfs utflow=1.11 cfs	
Pond DE-1: Drip Edge	Discarded=0.				Inflow=0.22 cf Dutflow=0.07 cf	
Pond DE-10: Drip Edge	Discarded=0.			•	Inflow=0.23 cf Dutflow=0.07 cf	
Pond DE-11: Drip Edge	Discarded=0.				Inflow=0.23 cf Dutflow=0.07 cf	
Pond DE-12: Drip Edge	Discarded=0.				Inflow=0.23 cf Dutflow=0.07 cf	
Pond DE-2: Drip Edge	Discarded=0.				Inflow=0.23 cf Dutflow=0.06 cf	
Pond DE-3: Drip Edge	Discarded=0.				Inflow=0.23 cf Dutflow=0.07 cf	
Pond DE-4: Drip Edge	Discarded=0.			-	Inflow=0.22 cf Dutflow=0.06 cf	
Pond DE-5: Drip Edge	Discarded=0.				Inflow=0.23 ct Dutflow=0.07 ct	

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Pond DE-6: Drip Edge	Peak Elev=0.74' Storage=0.003 af Inflow=0.23 cfs 761 cf Discarded=0.07 cfs 761 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 761 cf			
Pond DE-7: Drip Edge	Peak Elev=0.98' Storage=0.003 af Inflow=0.22 cfs 732 cf Discarded=0.06 cfs 732 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 732 cf			
Pond DE-8: Drip Edge	Peak Elev=0.68' Storage=0.002 af Inflow=0.22 cfs 732 cf Discarded=0.07 cfs 732 cf Primary=0.00 cfs 0 cf Outflow=0.07 cfs 732 cf			
Pond DE-9: Drip Edge	Peak Elev=1.23' Storage=0.003 af Inflow=0.23 cfs 758 cf Discarded=0.06 cfs 758 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 758 cf			
Pond GD-1: Ground Depression	Peak Elev=211.60' Storage=6 cf Inflow=0.31 cfs 3,302 cf Outflow=0.31 cfs 3,301 cf			
Pond GD-2: Ground Depression	Peak Elev=210.73' Storage=129 cf Inflow=0.32 cfs 3,097 cf Outflow=0.26 cfs 3,095 cf			
Pond GD-3: Ground Depression	Peak Elev=214.03' Storage=224 cf Inflow=0.37 cfs 1,221 cf Outflow=0.14 cfs 1,221 cf			
Pond GD-4: Ground Depression	Peak Elev=213.60' Storage=2 cf Inflow=0.09 cfs 628 cf Outflow=0.09 cfs 628 cf			
Pond IB-1: Infiltration Basin #1 Di	Peak Elev=211.35' Storage=1,631 cf Inflow=2.96 cfs 8,597 cf scarded=0.92 cfs 8,592 cf Primary=0.00 cfs 0 cf Outflow=0.92 cfs 8,592 cf			
Pond IB-2: Infiltration Basin #2	Peak Elev=213.02' Storage=8,680 cf Inflow=9.72 cfs 28,339 cf arded=1.73 cfs 28,314 cf Primary=0.00 cfs 0 cf Outflow=1.73 cfs 28,314 cf			
Pond IB-3: Infiltration Basin #3 Disca	Peak Elev=213.81' Storage=2,100 cf Inflow=4.03 cfs 11,681 cf arded=1.32 cfs 11,674 cf Primary=0.00 cfs 0 cf Outflow=1.32 cfs 11,674 cf			
Pond IB-4: Infiltration Basin #4 Disca	Peak Elev=212.33' Storage=6,059 cf Inflow=7.43 cfs 21,602 cf arded=1.52 cfs 21,587 cf Primary=0.00 cfs 0 cf Outflow=1.52 cfs 21,587 cf			
Total Runoff Area = 2,264,103 sf Runoff Volume = 98,430 cf Average Runoff Depth = 0.52" 94.00% Pervious = 2,128,182 sf 6.00% Impervious = 135,921 sf				

22-243 Post Development - Rev 2	Type III 24-hr	25 Year Rair	nfall=5.55"
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# Summary for Subcatchment PR-1A: South of stream

Runoff = 0.19 cfs @ 17.35 hrs, Volume= 3,102 cf, Depth> 0.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

_	A	rea (sf)	CN	Description			
	8	97,676	30	Woods, Go	od, HSG A		
		47,438	68	<50% Gras	s cover, Po	bor, HSG A	
		1,947	98	Paved park	ing, HSG A	\	
	9	947,061 32 Weighted Average					
	945,114 99.79% Pervious Area						
	1,947 0.21% Impervious Area				ervious Are	а	
	Tc (min)	Length (feet)			Capacity (cfs)	Description	
-	31.5	50	0.002	0 0.03		Sheet Flow, A-B	
	73.4	985	0.002	0 0.22		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow, B-C</b> Woodland Kv= 5.0 fps	
	101 0	1 005	Tatal				

104.9 1,035 Total

## Summary for Subcatchment PR-1B: North of stream

Runoff = 0.23 cfs @ 15.64 hrs, Volume= 4,182 cf, Depth> 0.07"

	Are	ea (sf)	CN E	<b>Description</b>					
	64	6,242	30 V	Woods, Good, HSG A					
	6	7,899	68 <	50% Gras	s cover, Po	or, HSG A			
33,279 30 Meadow, non-grazed, HSG A									
747,420 33 Weighted Average			Veighted A	verage					
	74	7,420	1	00.00% Pe	ervious Are	а			
Т	C I	Length	Slope	Velocity	Capacity	Description			
(mir	n)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.	.7	50	0.0200	0.15		Sheet Flow, A-B			
						Grass: Short n= 0.150 P2= 3.10"			
38.	.2	810	0.0050	0.35		Shallow Concentrated Flow, B-C			
						Woodland Kv= 5.0 fps			
43.	.9	860	Total						

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#### Summary for Subcatchment PR-1C: Jenkins - entrance

[49] Hint: Tc<2dt may require smaller dt

= 2.96 cfs @ 12.08 hrs, Volume= 8,597 cf, Depth> 3.16" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

Are	a (sf)	CN	Description				
ç	9,869	98	Paved road	s w/curbs 8	& sewers, HSG A		
19	9,113	68 ·	<50% Gras	s cover, Po	bor, HSG A		
3	3,635	98	Roofs, HSC	βA			
32	2,617	80	Weighted Average				
19	9,113	!	58.60% Pervious Area				
13	3,504	4	41.40% Impervious Area				
Tc L _(min)	ength (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
5.0					Direct Entry,		

#### Summary for Subcatchment PR-1D: Jenkins - Middle

[49] Hint: Tc<2dt may require smaller dt

Runoff 9.72 cfs @ 12.08 hrs, Volume= 28,339 cf, Depth> 3.26" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

Area (sf)	CN	Description	Description						
34,458	98	Paved road	ls w/curbs &	& sewers, HSG A					
60,702	68	<50% Gras	s cover, Po	bor, HSG A					
9,198	98	Roofs, HSC	Roofs, HSG A						
104,358	81	Weighted A	Weighted Average						
60,702		58.17% Pe	rvious Area	a					
43,656		41.83% Im	41.83% Impervious Area						
Tc Length	Slop	be Velocity	Capacity	Description					
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)						
5.0				Direct Entry,					

#### Summary for Subcatchment PR-1E: Gratuity Rd - middle near entrance

[49] Hint: Tc<2dt may require smaller dt

4.03 cfs @ 12.08 hrs, Volume= 11,681 cf, Depth> 3.07" Runoff =

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Are	ea (sf)	CN	Description				
1	5,364	98	Paved road	s w/curbs &	& sewers, HSG A		
2	8,536	68	<50% Gras	s cover, Po	oor, HSG A		
	1,785	98	Roofs, HSC	β A			
4	5,685	79	79 Weighted Average				
2	8,536		62.46% Pervious Area				
1	7,149	:	37.54% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	1		
5.0					Direct Entry,		

# Summary for Subcatchment PR-1F: Gratuity Rd - cul-de-sac

[49] Hint: Tc<2dt may require smaller dt

Runoff = 7.43 cfs @ 12.07 hrs, Volume= 21,602 cf, Depth> 3.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

A	rea (sf)	CN	Description				
	27,603	98	Paved road	s w/curbs &	& sewers, HSG A		
	42,235	68	<50% Gras	s cover, Po	oor, HSG A		
	7,414	98	Roofs, HSC	βA			
	77,252	82	Weighted Average				
	42,235		54.67% Pervious Area				
	35,017		45.33% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	1		
5.0			· · ·		Direct Entry,		

## Summary for Subcatchment PR-1G: (new Subcat)

Runoff = 0.31 cfs @ 12.65 hrs, Volume= 3,302 cf, Depth> 0.30"

 Area (sf)	CN	Description			
33,650	68	<50% Grass cover, Poor, HSG A			
 97,543	30	Woods, Good, HSG A			
131,193	40	Weighted Average			
131,193		100.00% Pervious Area			

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
16.5	50	0.0100	0.05	Y	Sheet Flow,				
8.3	250	0.0100	0.50		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps				
24.8	300	Total			· · · · · · · · · · · · · · · · · · ·				
	Summary for Subcatchment PR-1H: (new Subcat)								
Runoff	=	0.32 cfs	s@ 12.6	2 hrs, Volu	ume= 3,097 cf, Depth> 0.34"				
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr  25 Year Rainfall=5.55"								
Α	rea (sf)	CN D	escription						
	29,892				bor, HSG A				
-	77,876 07,768		Veighted A	od, HSG A					
	07,768			ervious Are	ea				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
16.5	50	0.0100	0.05		Sheet Flow,				
8.3	250	0.0100	0.50		Woods: Light underbrush n= 0.400 P2= 3.10" <b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps				
24.8	300	Total							
			Sum	mary for	r Subcatchment PR-1I:				
[49] Hint	: Tc<2dt	may requ	ire smaller	dt					
Runoff	=	0.37 cfs	s @ 12.1	0 hrs, Volu	ume= 1,221 cf, Depth> 1.07"				
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr  25 Year Rainfall=5.55"									
A	rea (sf)	CN D	escription						
	8,676				bor, HSG A				
	<u>5,002</u> 13,678		Voods, Go Veighted A	od, HSG A					
	13,678			ervious Are	ea				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

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#### Summary for Subcatchment PR-1K:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.09 cfs @ 12.27 hrs, Volume= 628 cf, Depth> 0.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

Ar	rea (sf)	CN	Description					
	5,678	68	<50% Gras	s cover, Po	oor, HSG A			
	11,311	30	Meadow, no	on-grazed, l	HSG A			
	16,989	43	Weighted Average					
	16,989		100.00% Pe	ervious Are	ea			
Tc	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)				
5.0					Direct Entry,			

#### Summary for Subcatchment PR-2: Gratuity Rd - entrance

Runoff	=	1.11 cfs @	12.14 hrs,	Volume=	3,673 cf, Depth> 2.43"
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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

_	A	rea (sf)	CN Description						
		2,680	98 F	Paved road	s w/curbs &	& sewers, HSG A			
_		15,434	68 <	<50% Gras	s cover, Po	or, HSG A			
		18,114	72 V	Veighted A	verage				
		15,434	8	35.20% Per	rvious Area				
		2,680	1	4.80% Imp	pervious Are	ea			
	Тс	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	7.5	50	0.0100	0.11		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.10"			
	2.3	98	0.0100	0.70		Shallow Concentrated Flow,			
_						Short Grass Pasture Kv= 7.0 fps			
	0 0	110	Total						

9.8 148 Total

#### Summary for Subcatchment Unit 1: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff 0.22 cfs @ 12.07 hrs, Volume= 732 cf, Depth> 4.92" =

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Area	sf) CN	Description						
1,7	85 98	98 Roofs, HSG A						
1,7	85	100.00% In	npervious A	rea				
	ngth Slop eet) (ft/		Capacity (cfs)	Description				
5.0				Direct Entry,				

### Summary for Subcatchment Unit 10: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.23 cfs @ 12.07 hrs, Volume= 761 cf, Depth> 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

Ar	ea (sf)	CN	Description						
	1,857	98	98 Roofs, HSG A						
	1,857		100.00% In	vrea					
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description				
5.0					Direct Entry,				

## Summary for Subcatchment Unit 11: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.23 cfs @ 12.07 hrs, Volume= 758 cf, Depth> 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

Ar	rea (sf)	CN	Description					
	1,850	98 Roofs, HSG A						
	1,850		100.00% In	npervious A	Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

### Summary for Subcatchment Unit 12: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.23 cfs @ 12.07 hrs, Volume= 761 cf, Depth> 4.92"

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A	rea (sf)	CN	Description						
	1,857	98	98 Roofs, HSG A						
	1,857		100.00% In	npervious A	Area				
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)					
5.0					Direct Entry,				

#### Summary for Subcatchment Unit 2: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.23 cfs @ 12.07 hrs, Volume= 758 cf, Depth> 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

A	rea (sf)	CN Description							
	1,850	98	98 Roofs, HSG A						
	1,850	100.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description				
5.0					Direct Entry,				

## Summary for Subcatchment Unit 3: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.23 cfs @ 12.07 hrs, Volume= 761 cf, Depth> 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

Are	ea (sf)	CN E	Description						
	1,857	98 F	98 Roofs, HSG A						
	1,857	1	rea						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

## Summary for Subcatchment Unit 4: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.22 cfs @ 12.07 hrs, Volume=

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

A	rea (sf)	CN	Description					
	1,785	98	Roofs, HSC	βA				
	1,785	35 100.00% Impervious Area						
Tc (min)	Length (feet)	Slop (ft/fl		Capacity (cfs)	Description			
5.0	()	(101)	., (	(0.0)	Direct Entry,			
		_	_	_				

#### Summary for Subcatchment Unit 5: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.23 cfs @ 12.07 hrs, Volume= 758 cf, Depth> 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

A	rea (sf)	CN I	Description						
	1,850	98	98 Roofs, HSG A						
	1,850	100.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0					Direct Entry,				

## Summary for Subcatchment Unit 6: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.23 cfs @ 12.07 hrs, Volume= 761 cf, Depth> 4.92"

A	rea (sf)	CN I	Description					
	1,857	98 Roofs, HSG A						
	1,857		100.00% In	npervious A	Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)				
5.0					Direct Entry,			

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### Summary for Subcatchment Unit 7: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.22 cfs @ 12.07 hrs, Volume= 732 cf, Depth> 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

_	A	rea (sf)	CN	Description			
		1,785	98	Roofs, HSG A			
-		1,785		100.00% In	npervious A	Area	
	Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description	
-	5.0					Direct Entry,	

### Summary for Subcatchment Unit 8: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.22 cfs @ 12.07 hrs, Volume= 732 cf, Depth> 4.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 Year Rainfall=5.55"

ea (sf)	CN E	Description		
1,785	98 F	Roofs, HSG	βA	
1,785	1	00.00% Im	npervious A	rea
			<b>-</b>	
Length	Slope	Velocity	Capacity	Description
(feet)	(ft/ft)	(ft/sec)	(cfs)	
				Direct Entry,
	1,785 1,785 Length	1,785 98 F 1,785 1 Length Slope	1,785         98         Roofs, HSG           1,785         100.00%         Im           Length         Slope         Velocity	1,78598Roofs, HSG A1,785100.00%Impervious ALengthSlopeVelocityCapacity

#### Summary for Subcatchment Unit 9: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.23 cfs @ 12.07 hrs, Volume= 758 cf, Depth> 4.92"

 Area (sf)	CN	Description
1,850	98	Roofs, HSG A
1,850		100.00% Impervious Area

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Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.0					Direct Entry,

## Summary for Reach DP-1: Stream

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	1,694,481 sf,	0.11% Impervious,	Inflow Depth >	0.05"	for 25 Year event
Inflow	=	0.39 cfs @	16.45 hrs, Volume=	7,285 cf		
Outflow	=	0.39 cfs @	16.45 hrs, Volume=	7,285 cf	, Atten	= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

# Summary for Reach DP-2: Gratuity Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	18,114 sf,	14.80% Impervious,	Inflow Depth >	2.43"	for 25 Year event
Inflow	=	1.11 cfs @	12.14 hrs, Volume=	3,673 c	f	
Outflow	=	1.11 cfs @	12.14 hrs, Volume=	3,673 c	f, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

## Summary for Pond DE-1: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage

Inflow Area =	1,785 sf,100.00% Impervious,	Inflow Depth > 4.92" for 25 Year event
Inflow =	0.22 cfs @ 12.07 hrs, Volume=	732 cf
Outflow =	0.07 cfs @ 11.85 hrs, Volume=	732 cf, Atten= 67%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.85 hrs, Volume=	732 cf
Primary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.68' @ 12.34 hrs Surf.Area= 0.009 ac Storage= 0.002 af

Plug-Flow detention time= 6.4 min calculated for 732 cf (100% of inflow) Center-of-Mass det. time= 6.2 min (740.0 - 733.8)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	0.007 af	<b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids
Device	Routing	Invert O	utlet Devices
#1	Discarded	0.00' <b>8.</b>	270 in/hr Exfiltration over Surface area
#2	Primary	H 2.	2.0' long x 2.0' breadth Broad-Crested Rectangular Weir           ead (feet)         0.20         0.40         0.60         0.80         1.00         1.20         1.40         1.60         1.80         2.00           50         3.00         3.50         0         0         0.61         2.60         2.66         2.70         2.77         2.89         2.88

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2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.07 cfs @ 11.85 hrs HW=0.02' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Summary for Pond DE-10: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1) Inflow Area = 1.857 sf,100.00% Impervious, Inflow Depth > 4.92" for 25 Year event Inflow = 0.23 cfs @ 12.07 hrs, Volume= 761 cf Outflow = 0.07 cfs @ 11.85 hrs, Volume= 761 cf, Atten= 68%, Lag= 0.0 min Discarded = 0.07 cfs @ 11.85 hrs, Volume= 761 cf 5.00 hrs, Volume= Primary 0.00 cfs @ 0 cf = Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.74' @ 12.36 hrs Surf.Area= 0.009 ac Storage= 0.003 af Plug-Flow detention time= 7.1 min calculated for 761 cf (100% of inflow) Center-of-Mass det. time= 6.9 min (740.7 - 733.8)

Volume	Invert	Avail.Storag	e Storage Description
#1	0.00'	0.007 a	af <b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids
Device	Routing	Invert (	Outlet Devices
#1	Discarded	0.00' 8	8.270 in/hr Exfiltration over Surface area
#2	Primary	 2 (	<b>72.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.07 cfs @ 11.85 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Summary for Pond DE-11: Drip Edge

- [82] Warning: Early inflow requires earlier time span
- [92] Warning: Device #2 is above defined storage

22-243 Post Development - Rev 2Type III 24-hr25 Year Rainfall=5.55"Prepared by Landtech Consultants, Inc.Printed1/14/2025HydroCAD® 10.00-26 s/n 00798 © 2020 HydroCAD Software Solutions LLCPage 73			
Inflow Area =       1,850 sf,100.00% Impervious, Inflow Depth > 4.92" for 25 Year event         Inflow =       0.23 cfs @ 12.07 hrs, Volume=       758 cf         Outflow =       0.07 cfs @ 11.80 hrs, Volume=       758 cf, Atten= 70%, Lag= 0.0 min         Discarded =       0.07 cfs @ 11.80 hrs, Volume=       758 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf			
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.88' @ 12.38 hrs Surf.Area= 0.008 ac Storage= 0.003 af			
Plug-Flow detention time= 8.5 min calculated for 756 cf (100% of inflow) Center-of-Mass det. time= 8.3 min ( 742.1 - 733.8 )			
Volume Invert Avail.Storage Storage Description			
#1 0.00' 0.007 af <b>3.00'W x 120.00'L x 2.00'H Prismatoid</b> 0.017 af Overall x 40.0% Voids			
Device Routing Invert Outlet Devices			
#1       Discarded       0.00'       8.270 in/hr Exfiltration over Surface area         #2       Primary       2.00'       72.0' long x 2.0' breadth Broad-Crested Rectangular Weir         Head (feet)       0.20       0.40       0.60       0.80       1.00       1.20       1.40       1.60       1.80       2.00'         2.50       3.00       3.50       Coef. (English)       2.54       2.61       2.61       2.60       2.66       2.70       2.77       2.89       2.88         2.85       3.07       3.20       3.32			
<b>Discarded OutFlow</b> Max=0.07 cfs @ 11.80 hrs HW=0.02' (Free Discharge) -1=Exfiltration (Exfiltration Controls 0.07 cfs)			
Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)			
Summary for Pond DE-12: Drip Edge			
[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)			
Inflow Area =       1,857 sf,100.00% Impervious, Inflow Depth > 4.92" for 25 Year event         Inflow =       0.23 cfs @ 12.07 hrs, Volume=       761 cf         Outflow =       0.07 cfs @ 11.85 hrs, Volume=       761 cf, Atten= 68%, Lag= 0.0 min         Discarded =       0.07 cfs @ 11.85 hrs, Volume=       761 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf			
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.74' @ 12.36 hrs Surf.Area= 0.009 ac Storage= 0.003 af			
Plug-Flow detention time= 7.1 min calculated for 761 cf (100% of inflow) Center-of-Mass det. time= 6.9 min(740.7-733.8)			

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 Type III 24-hr
 25 Year Rainfall=5.55"

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Volume	Invert	Avail.Storage	Storage Description		
#1	0.00'	0.007 af	<b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids		
Device	Routing	Invert Ou	utlet Devices		
#1 #2	Discarded Primary	2.00' <b>72</b> He 2.9 Co	<b>270 in/hr Exfiltration over Surface area</b> <b>.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 50 3.00 3.50 bef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 35 3.07 3.20 3.32		
		Max=0.07 cfs @ filtration Control	211.85 hrs HW=0.03' (Free Discharge) s 0.07 cfs)		
			.00 hrs HW=0.00' (Free Discharge) eir (Controls 0.00 cfs)		
		Sum	mary for Pond DE-2: Drip Edge		
[92] Wai	[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)				
Inflow A Inflow Outflow Discarde Primary	= 0. = 0. ed = 0.	1,850 sf,100.0 23 cfs @ 12.07 06 cfs @ 11.80 06 cfs @ 11.80 00 cfs @ 5.00	) hrs, Volume= 758 cf, Atten= 73%, Lag= 0.0 min ) hrs, Volume= 758 cf		
			an= 5.00-20.00 hrs, dt= 0.05 hrs area= 0.008 ac Storage= 0.003 af		
		me= 10.4 min ca me= 10.3 min (	alculated for 756 cf (100% of inflow) 744.1 - 733.8)		
Volume	Invert	Avail.Storage	Storage Description		
#1	0.00'	0.006 af	<b>3.00'W x 110.00'L x 2.00'H Prismatoid</b> 0.015 af Overall x 40.0% Voids		
Device	Routing		utlet Devices		
#1 #2	Discarded Primary	2.00' <b>72</b> He 2.9 Co	<b>270 in/hr Exfiltration over Surface area</b> <b>1.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 50 3.00 3.50 pef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 35 3.07 3.20 3.32		

**Discarded OutFlow** Max=0.06 cfs @ 11.80 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## Summary for Pond DE-3: Drip Edge

[92] Warning: Device #2 is above defined storage

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area =	1,857 sf,100.00% Impervious,	Inflow Depth > 4.92" for 25 Year event
Inflow =	0.23 cfs @ 12.07 hrs, Volume=	761 cf
Outflow =	0.07 cfs @ 11.80 hrs, Volume=	761 cf, Atten= 72%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.80 hrs, Volume=	761 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.97' @ 12.40 hrs Surf.Area= 0.008 ac Storage= 0.003 af

Plug-Flow detention time= 9.5 min calculated for 759 cf (100% of inflow) Center-of-Mass det. time= 9.3 min (743.1 - 733.8)

Volume	Invert	Avail.Storag	ge Storage Description
#1	0.00'	0.006	af <b>3.00'W x 115.00'L x 2.00'H Prismatoid</b> 0.016 af Overall x 40.0% Voids
Device	Routing	Invert	Outlet Devices
#1	Discarded		8.270 in/hr Exfiltration over Surface area
#2	Primary		<b>72.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.07 cfs @ 11.80 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond DE-4: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

<b>22-243 Post Development - Rev 2</b> Type III 24-hr25 Year Rainfall=5.55"Prepared by Landtech Consultants, Inc.Printed1/14/2025HydroCAD® 10.00-26 s/n 00798 © 2020 HydroCAD Software Solutions LLCPage 76					
Inflow Area =       1,785 sf,100.00% Impervious, Inflow Depth > 4.92" for 25 Year event         Inflow =       0.22 cfs @ 12.07 hrs, Volume=       732 cf         Outflow =       0.06 cfs @ 11.75 hrs, Volume=       732 cf, Atten= 74%, Lag= 0.0 min         Discarded =       0.06 cfs @ 11.75 hrs, Volume=       732 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf					
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.19' @ 12.43 hrs Surf.Area= 0.007 ac Storage= 0.003 af					
Plug-Flow detention time= 12.0 min calculated for 732 cf (100% of inflow) Center-of-Mass det. time= 11.8 min ( 745.6 - 733.8 )					
Volume Invert Avail.Storage Storage Description					
#1 0.00' 0.006 af <b>3.00'W x 100.00'L x 2.00'H Prismatoid</b> 0.014 af Overall x 40.0% Voids					
Device Routing Invert Outlet Devices					
#1       Discarded         #2       Primary         2.00'       8.270 in/hr Exfiltration over Surface area         72.0' long x 2.0' breadth Broad-Crested Rectangular Weir         Head (feet)       0.20         0.50       3.00         2.50       3.00         0.50       Coef. (English)         2.54       2.61         2.65       3.07         3.20       3.32					
Discarded OutFlow Max=0.06 cfs @ 11.75 hrs HW=0.02' (Free Discharge)					
Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) <sup>●</sup> 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)					
Summary for Pond DE-5: Drip Edge					
[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage					
Inflow Area =       1,850 sf,100.00% Impervious, Inflow Depth > 4.92" for 25 Year event         Inflow =       0.23 cfs @ 12.07 hrs, Volume=       758 cf         Outflow =       0.07 cfs @ 11.80 hrs, Volume=       758 cf, Atten= 70%, Lag= 0.0 min         Discarded =       0.07 cfs @ 11.80 hrs, Volume=       758 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf					
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.88' @ 12.38 hrs Surf.Area= 0.008 ac Storage= 0.003 af					
Plug-Flow detention time= 8.5 min calculated for 756 cf (100% of inflow) Center-of-Mass det. time= 8.3 min(742.1 - 733.8)					
Volume Invert Avail.Storage Storage Description					
#1 0.00' 0.007 af <b>3.00'W x 120.00'L x 2.00'H Prismatoid</b> 0.017 af Overall x 40.0% Voids					

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Device	Routing	Invert	Outlet Devices			
#1 #2	Discarded Primary	2.00'	8.270 in/hr Exfiltration over Surface area 72.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32			
		Max=0.07 cfs filtration Contr	@ 11.80 hrs HW=0.02' (Free Discharge) rols 0.07 cfs)			
			5.00 hrs HW=0.00' (Free Discharge) <b>Veir</b> (Controls 0.00 cfs)			
		Su	mmary for Pond DE-6: Drip Edge			
[92] Wa	rning: Device	#2 is above de	earlier time span efined storage ire smaller dt or Finer Routing (severity=1)			
Inflow A Inflow Outflow Discarde Primary	= 0. = 0. ed = 0.	23 cfs @ 12.0 07 cfs @ 11.0 07 cfs @ 11.0	0.00% Impervious, Inflow Depth > 4.92" for 25 Year event 07 hrs, Volume= 761 cf 85 hrs, Volume= 761 cf, Atten= 68%, Lag= 0.0 min 85 hrs, Volume= 761 cf 00 hrs, Volume= 0 cf			
			Span= 5.00-20.00 hrs, dt= 0.05 hrs f.Area= 0.009 ac Storage= 0.003 af			
			calculated for 761 cf (100% of inflow) 740.7 - 733.8)			
Volume	Invert	Avail.Storad	e Storage Description			
#1	0.00'	0.007 a				
Device	Routing	Invert (	Outlet Devices			
#1 #2	Discarded Primary	0.00' 2.00'	8.270 in/hr Exfiltration over Surface area         72.0' long x 2.0' breadth Broad-Crested Rectangular Weir         Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00         2.50 3.00 3.50         Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88         2.85 3.07 3.20 3.32			
		Max=0.07 cfs filtration Contr	@ 11.85 hrs HW=0.03' (Free Discharge) rols 0.07 cfs)			
		<b>Primary OutFlow</b> Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge)				

**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

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# Summary for Pond DE-7: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area =	1,785 sf,100.00% Impervious,	Inflow Depth > 4.92" for 25 Year event
Inflow =	0.22 cfs @ 12.07 hrs, Volume=	732 cf
Outflow =	0.06 cfs @ 11.80 hrs, Volume=	732 cf, Atten= 72%, Lag= 0.0 min
Discarded =	0.06 cfs @ 11.80 hrs, Volume=	732 cf
Primary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0 cf
Tilliary –		0.01

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.98' @ 12.40 hrs Surf.Area= 0.008 ac Storage= 0.003 af

Plug-Flow detention time= 9.6 min calculated for 732 cf (100% of inflow) Center-of-Mass det. time= 9.4 min (743.2 - 733.8)

Volume	Invert	Avail.Storage	e Storage Description
#1	0.00'	0.006 a	f <b>3.00'W x 110.00'L x 2.00'H Prismatoid</b> 0.015 af Overall x 40.0% Voids
Device	Routing	Invert C	Dutlet Devices
#1	Discarded	0.00' <b>8</b>	3.270 in/hr Exfiltration over Surface area
#2	Primary	F 2 C	<b>2.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> lead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.06 cfs @ 11.80 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## Summary for Pond DE-8: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage

Inflow Area =	1,785 sf,100.00% Impervious,	Inflow Depth > 4.92" for 25 Year event
Inflow =	0.22 cfs @ 12.07 hrs, Volume=	732 cf
Outflow =	0.07 cfs @ 11.85 hrs, Volume=	732 cf, Atten= 67%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.85 hrs, Volume=	732 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 0.68' @ 12.34 hrs Surf.Area= 0.009 ac Storage= 0.002 af

Plug-Flow detention time= 6.4 min calculated for 732 cf (100% of inflow)

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Center-of-Mass det. time= 6.2 min (740.0 - 733.8)

Volume	Invert	Avail.Storage	Storage Description		
#1	0.00'	0.007 af			
Device	Routing	Invert O	utlet Devices		
#1 #2	Discarded Primary	2.00' <b>72</b> He 2. Ce	8.270 in/hr Exfiltration over Surface area 72.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32		
Ê—1=Ex	filtration (Exi	filtration Contro	0 11.85 hrs HW=0.02' (Free Discharge) Is 0.07 cfs) 5.00 hrs HW=0.00' (Free Discharge)		
			eir (Controls 0.00 cfs)		
		Sum	mary for Pond DE-9: Drip Edge		
[92] Wai	rning: Device a	low requires ea #2 is above defi ons may require			
Inflow A Inflow Outflow Discarde Primary	= 0.2 = 0.0 ed = 0.0	23 cfs @ 12.07 )6 cfs @ 11.75 )6 cfs @ 11.75	00% Impervious, Inflow Depth > 4.92" for 25 Year event 7 hrs, Volume= 758 cf 5 hrs, Volume= 758 cf, Atten= 75%, Lag= 0.0 min 5 hrs, Volume= 758 cf 0 hrs, Volume= 0 cf		
			an= 5.00-20.00 hrs, dt= 0.05 hrs Area= 0.007 ac Storage= 0.003 af		
			alculated for 758 cf (100% of inflow) 746.1 - 733.8)		
Volume	Invert	Avail.Storage			
#1	0.00'	0.006 af	<b>3.00'W x 102.00'L x 2.00'H Prismatoid</b> 0.014 af Overall x 40.0% Voids		
Device	Routing	Invert O	utlet Devices		
#1 #2	Discarded Primary	2.00' <b>72</b> He 2. Ce	<b>270 in/hr Exfiltration over Surface area</b> <b>2.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 50 3.00 3.50 oef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 85 3.07 3.20 3.32		

**Discarded OutFlow** Max=0.06 cfs @ 11.75 hrs HW=0.02' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond GD-1: Ground Depression

Inflow Area =	131,193 sf, 0.00% Impervi	ous, Inflow Depth > 0.30"	for 25 Year event
Inflow =	0.31 cfs @ 12.65 hrs, Volun	ne= 3,302 cf	
Outflow =	0.31 cfs @ 12.65 hrs, Volun	ne= 3,301 cf, Atte	n= 0%, Lag= 0.3 min
Discarded =	0.31 cfs @ 12.65 hrs, Volun	ne= 3,301 cf	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 211.60' @ 12.65 hrs Surf.Area= 1,628 sf Storage= 6 cf

Plug-Flow detention time= 0.3 min calculated for 3,290 cf (100% of inflow) Center-of-Mass det. time= 0.2 min (908.6 - 908.3)

Volume	Inver	t Avail.St	orage	Storage De	escription	
#1	211.60	' 2,9	903 cf	Custom St	tage Data (Pri	ismatic) Listed below (Recalc)
Elevatio (fee		urf.Area (sq-ft)	Inc.: (cubic	Store -feet)	Cum.Store (cubic-feet)	
211.6	60	1,514		0	0	
212.0	0	13,000	2	2,903	2,903	
Device	Routing	Inver	t Outle	t Devices		
#1	Discarded	211.60	8.270	in/hr Exfil	tration over S	Surface area

**Discarded OutFlow** Max=0.31 cfs @ 12.65 hrs HW=211.60' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.31 cfs)

## Summary for Pond GD-2: Ground Depression

Inflow Area =	107,768 sf, 0.00% Impervious,	Inflow Depth > 0.34" for 25 Year event
Inflow =	0.32 cfs @ 12.62 hrs, Volume=	3,097 cf
Outflow =	0.26 cfs @ 12.84 hrs, Volume=	3,095 cf, Atten= 19%, Lag= 13.3 min
Discarded =	0.26 cfs @ 12.84 hrs, Volume=	3,095 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 210.73' @ 12.84 hrs Surf.Area= 1,373 sf Storage= 129 cf

Plug-Flow detention time= 3.0 min calculated for 3,085 cf (100% of inflow) Center-of-Mass det. time= 2.8 min (904.2 - 901.4)

Volume	Invert	Avail.Storage	Storage Description
#1	210.60'	3,547 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
210.60	550	0	0
211.00	2,998	710	710
211.50	8,352	2,838	3,547

Device	Routing	Invert	Outlet Devices
#1	Discarded	210.60'	8.270 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.26 cfs @ 12.84 hrs HW=210.73' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.26 cfs)

#### Summary for Pond GD-3: Ground Depression

Inflow Area =	13,678 sf, 0.00% Impervious,	Inflow Depth > 1.07" for 25 Year event
Inflow =	0.37 cfs @ 12.10 hrs, Volume=	1,221 cf
Outflow =	0.14 cfs @ 12.45 hrs, Volume=	1,221 cf, Atten= 62%, Lag= 21.1 min
Discarded =	0.14 cfs @ 12.45 hrs, Volume=	1,221 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 214.03' @ 12.45 hrs Surf.Area= 750 sf Storage= 224 cf

Plug-Flow detention time= 11.3 min calculated for 1,221 cf (100% of inflow) Center-of-Mass det. time= 11.1 min ( 846.8 - 835.7 )

Volume	Invert	Avail.Sto	rage Sto	rage Description	
#1	213.60'	9	19 cf Cus	stom Stage Data (P	rismatic) Listed below (Recalc)
Elevation (feet)		ırf.Area (sq-ft)	Inc.Stor (cubic-fee	• • • • • • • • • •	
213.60		355		0 0	
214.00		660	20	)3 203	
214.50		2,205	71	919	
Device I	Routing	Invert	Outlet De	evices	
#1 I	Discarded	213.60'	8.270 in/	hr Exfiltration over	Surface area

**Discarded OutFlow** Max=0.14 cfs @ 12.45 hrs HW=214.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.14 cfs)

### Summary for Pond GD-4: Ground Depression

Inflow Area =	16,989 sf, 0.00% Impervious	, Inflow Depth > 0.44" for 25 Year event
Inflow =	0.09 cfs @ 12.27 hrs, Volume=	628 cf
Outflow =	0.09 cfs @ 12.27 hrs, Volume=	628 cf, Atten= 0%, Lag= 0.0 min
Discarded =	0.09 cfs @ 12.27 hrs, Volume=	628 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.60' @ 12.27 hrs Surf.Area= 556 sf Storage= 2 cf

Plug-Flow detention time= 0.3 min calculated for 626 cf (100% of inflow)

Center-of-Mass det. time= 0.3 min (876.6 - 876.3)

Volume	Invert	Avail.Sto	orage St	orage De	escription	
#1	213.60'	3	63 cf <b>C</b> ı	ustom St	age Data (Pri	smatic) Listed below (Recalc)
Elevatio (fee		ırf.Area (sq-ft)	Inc.Ste (cubic-fe		Cum.Store (cubic-feet)	
213.6	0	550		0	0	
214.0	0	1,263	3	863	363	
Device	Routing	Invert	Outlet E	Devices		
#1	Discarded	213.60'	8.270 ir	n/hr Exfil	tration over S	Surface area

**Discarded OutFlow** Max=0.11 cfs @ 12.27 hrs HW=213.60' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.11 cfs)

### Summary for Pond IB-1: Infiltration Basin #1

Inflow Area =	32,617 sf, 41.40% Impervious,	Inflow Depth > 3.16" for 25 Year event
Inflow =	2.96 cfs @ 12.08 hrs, Volume=	8,597 cf
Outflow =	0.92 cfs @ 12.40 hrs, Volume=	8,592 cf, Atten= 69%, Lag= 19.6 min
Discarded =	0.92 cfs @ 12.40 hrs, Volume=	8,592 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 211.35' @ 12.40 hrs Surf.Area= 4,815 sf Storage= 1,631 cf

Plug-Flow detention time= 10.2 min calculated for 8,563 cf (100% of inflow) Center-of-Mass det. time= 9.9 min (792.5 - 782.6)

Volume	Inve	ert Avail	.Storage	Storage D	Description	
#1	211.0	)0' 1	1,236 cf	Custom S	Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio	n	Surf.Area	Inc	.Store	Cum.Store	
(fee	t)	(sq-ft)	(cubi	c-feet)	(cubic-feet)	
211.0	0	4,480		0	0	
212.0	0	5,436		4,958	4,958	
213.0	0	7,119		6,278	11,236	
Device	Routing	Inv	ert Outl	et Devices		
#1	Primary	212.				oad-Crested Rectangular Weir
				· · ·		0.80 1.00 1.20 1.40 1.60
			Coe	f. (English)	2.68 2.70 2.	70 2.64 2.63 2.64 2.64 2.63

#2 Discarded 211.00' 8.270 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.92 cfs @ 12.40 hrs HW=211.35' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.92 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=211.00' (Free Discharge) ←1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Summary for Pond IB-2: Infiltration Basin #2

Inflow Area =	104,358 sf, 41.83% Impervious,	Inflow Depth > 3.26" for 25 Year event
Inflow =	9.72 cfs @ 12.08 hrs, Volume=	28,339 cf
Outflow =	1.73 cfs @ 12.54 hrs, Volume=	28,314 cf, Atten= 82%, Lag= 28.0 min
Discarded =	1.73 cfs @ 12.54 hrs, Volume=	28,314 cf
Primary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.02' @ 12.54 hrs Surf.Area= 9,061 sf Storage= 8,680 cf

Plug-Flow detention time= 35.4 min calculated for 28,314 cf (100% of inflow) Center-of-Mass det. time= 35.0 min (815.4 - 780.4)

Volume	Inver	t Avail.Sto	rage Storage	e Description	
#1	212.00	' 28,93	39 cf Custon	n Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio	et)	urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
212.0	-	7,901	0	0	
213.0	-	9,033	8,467	8,467	
214.0	00	10,222	9,628	18,095	
215.0	00	11,467	10,845	28,939	
Device	Routing	Invert	Outlet Device	es	
#1	Discarded	212.00'	8.270 in/hr E	xfiltration over	Surface area
#2	Primary	214.00'	<b>5.0' long x 1</b> Head (feet)	5.0' breadth Bro	<b>Dad-Crested Rectangular Weir</b> 0.80 1.00 1.20 1.40 1.60 70 2.64 2.63 2.64 2.64 2.63
Discard	<b>Discarded OutFlow</b> Max=1.73 cfs @ 12.54 hrs HW=213.02' (Free Discharge)				

**1=Exfiltration** (Exfiltration Controls 1.73 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=212.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## Summary for Pond IB-3: Infiltration Basin #3

Inflow Area =	45,685 sf, 37.54% Impervious,	Inflow Depth > 3.07" for 25 Year event
Inflow =	4.03 cfs @ 12.08 hrs, Volume=	11,681 cf
Outflow =	1.32 cfs @ 12.39 hrs, Volume=	11,674 cf, Atten= 67%, Lag= 18.8 min
Discarded =	1.32 cfs @ 12.39 hrs, Volume=	11,674 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.81' @ 12.39 hrs Surf.Area= 6,870 sf Storage= 2,100 cf

Plug-Flow detention time= 9.2 min calculated for 11,635 cf (100% of inflow) Center-of-Mass det. time= 8.9 min (793.6 - 784.7)

Prepare	ed by Landte	lopment - I ech Consulta s/n 00798 ©	Type III 24-hr         25 Year Rainfall=5.55"           Printed         1/14/2025           ns LLC         Page 84		
Volume	Invert	Avail.Stor	age Storage	e Description	
#1	213.50'		<u> </u>		smatic) Listed below (Recalc)
Elevatio (fee		rf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
	213.50 6,490 214.00 7,094		0 3,396	0 3,396	
-	214.50 7,710		3,701	7,097	
215.0 215.5	215.00 8,343		4,013 4,571	11,110 15,681	
210.0	0	9,940	4,571	15,001	
Device	Routing	Invert	Outlet Device	es	
#1	Discarded	213.50'	8.270 in/hr Exfiltration over S		
#2	Primary	214.50'	5.0' long x 3.0' breadth Broad-Crested Rectangular Weir		
					0.80 1.00 1.20 1.40 1.60 1.80 2.00
				.50 4.00 4.50	SO 267 265 264 264 269 269
				.92 2.97 3.07 3.	38 2.67 2.65 2.64 2.64 2.68 2.68
			_		-

**Discarded OutFlow** Max=1.32 cfs @ 12.39 hrs HW=213.81' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.32 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=213.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

## Summary for Pond IB-4: Infiltration Basin #4

Inflow Area =	77,252 sf, 45.33% Impervious,	Inflow Depth > 3.36" for 25 Year event
Inflow =	7.43 cfs @ 12.07 hrs, Volume=	21,602 cf
Outflow =	1.52 cfs @ 12.51 hrs, Volume=	21,587 cf, Atten= 80%, Lag= 26.0 min
Discarded =	1.52 cfs @ 12.51 hrs, Volume=	21,587 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 212.33' @ 12.51 hrs Surf.Area= 7,949 sf Storage= 6,059 cf

Plug-Flow detention time= 26.7 min calculated for 21,587 cf (100% of inflow) Center-of-Mass det. time= 26.4 min (804.5 - 778.1)

Volume	Invert	Avai	il.Storage	Storage	e Description	
#1	211.50'		21,638 cf	Custor	n Stage Data (Pr	ismatic) Listed below (Recalc)
Elevation (feet)		.Area sq-ft)		.Store c-feet)	Cum.Store (cubic-feet)	
211.50		6,864	•	0	0	
212.00	-	7,370		3,559	3,559	
213.00	ę	9,144		8,257	11,816	
214.00	10	0,501		9,823	21,638	

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Device	Routing	Invert	Outlet Devices
#1	Discarded	211.50'	8.270 in/hr Exfiltration over Surface area
#2	Primary	213.00'	5.0' long x 15.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63

**Discarded OutFlow** Max=1.52 cfs @ 12.51 hrs HW=212.33' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.52 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=211.50' (Free Discharge) —2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

22-243 Post Development - Rev 2Type III 24-hr 100 Year Rainfall=7.80"Prepared by Landtech Consultants, Inc.Printed 1/14/2025HydroCAD® 10.00-26 s/n 00798 © 2020 HydroCAD Software Solutions LLCPage 86
Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method
Subcatchment PR-1A: South of streamRunoff Area=947,061 sf0.21% ImperviousRunoff Depth>0.38"Flow Length=1,035'Slope=0.0020 '/'Tc=104.9 minCN=32Runoff=1.71 cfs29,893 cf
Subcatchment PR-1B: North of streamRunoff Area=747,420 sf0.00% ImperviousRunoff Depth>0.47"Flow Length=860'Tc=43.9 minCN=33Runoff=2.42 cfs29,303 cf
Subcatchment PR-1C: Jenkins - entrance Runoff Area=32,617 sf 41.40% Impervious Runoff Depth>5.12" Tc=5.0 min CN=80 Runoff=4.73 cfs 13,917 cf
Subcatchment PR-1D: Jenkins - Middle Runoff Area=104,358 sf 41.83% Impervious Runoff Depth>5.23" Tc=5.0 min CN=81 Runoff=15.41 cfs 45,516 cf
Subcatchment PR-1E: Gratuity Rd - middle Runoff Area=45,685 sf 37.54% Impervious Runoff Depth>5.01" Tc=5.0 min CN=79 Runoff=6.50 cfs 19,060 cf
Subcatchment PR-1F: Gratuity Rd -Runoff Area=77,252 sf 45.33% Impervious Runoff Depth>5.35"Tc=5.0 min CN=82 Runoff=11.60 cfs 34,429 cf
Subcatchment PR-1G: (new Subcat) Flow Length=300' Slope=0.0100 '/' Tc=24.8 min CN=40 Runoff=1.77 cfs 11,031 cf
Subcatchment PR-1H: (new Subcat)Runoff Area=107,768 sf0.00% ImperviousRunoff Depth>1.09"Flow Length=300'Slope=0.0100 '/'Tc=24.8 minCN=41Runoff=1.63 cfs9,814 cf
Subcatchment PR-11:Runoff Area=13,678 sf0.00% ImperviousRunoff Depth>2.32"Tc=5.0 minCN=54Runoff=0.89 cfs2,645 cf
Subcatchment PR-1K:Runoff Area=16,989 sf0.00% ImperviousRunoff Depth>1.28"Tc=5.0 minCN=43Runoff=0.52 cfs1,812 cf
Subcatchment PR-2: Gratuity Rd - entrance Runoff Area=18,114 sf 14.80% Impervious Runoff Depth>4.22" Flow Length=148' Slope=0.0100 '/' Tc=9.8 min CN=72 Runoff=1.91 cfs 6,365 cf
Subcatchment Unit 1: Unit A - BirchRunoff Area=1,785 sf100.00% ImperviousRunoff Depth>6.96"Tc=5.0 minCN=98Runoff=0.32 cfs1,036 cf
Subcatchment Unit 10: Unit C - Hickory Runoff Area=1,857 sf 100.00% Impervious Runoff Depth>6.96" Tc=5.0 min CN=98 Runoff=0.33 cfs 1,078 cf
Subcatchment Unit 11: Unit B - Hemlock Runoff Area=1,850 sf 100.00% Impervious Runoff Depth>6.96" Tc=5.0 min CN=98 Runoff=0.33 cfs 1,074 cf
Subcatchment Unit 12: Unit C - Hickory Runoff Area=1,857 sf 100.00% Impervious Runoff Depth>6.96" Tc=5.0 min CN=98 Runoff=0.33 cfs 1,078 cf
Subcatchment Unit 2: Unit B - Hemlock Runoff Area=1,850 sf 100.00% Impervious Runoff Depth>6.96" Tc=5.0 min CN=98 Runoff=0.33 cfs 1,074 cf

<b>22-243 Post Developme</b> Prepared by Landtech Cor HydroCAD® 10.00-26 s/n 0075	sultants, Inc.	CAD Software Solution	Type III 24-hr 10	Printed 1/2	
Subcatchment Unit 3: Unit 0	C - Hickory	Runoff Area=1,857 s To	f 100.00% Impervic =5.0 min CN=98 F		
Subcatchment Unit 4: Unit A	A - Birch	Runoff Area=1,785 s To	f 100.00% Impervic =5.0 min CN=98 F		
Subcatchment Unit 5: Unit E	3 - Hemlock	Runoff Area=1,850 s To	f 100.00% Impervic =5.0 min CN=98 F		
Subcatchment Unit 6: Unit 0	C - Hickory	Runoff Area=1,857 s To	f 100.00% Impervic =5.0 min CN=98 F		
Subcatchment Unit 7: Unit A	A - Birch	Runoff Area=1,785 s To	f 100.00% Impervic =5.0 min CN=98 F		
Subcatchment Unit 8: Unit A	A - Birch	Runoff Area=1,785 s To	f 100.00% Impervic =5.0 min CN=98 F		
Subcatchment Unit 9: Unit E	3 - Hemlock	Runoff Area=1,850 s To	f 100.00% Impervic =5.0 min CN=98 F		
Reach DP-1: Stream				nflow=3.22 cfs 5 tflow=3.22 cfs 5	
Reach DP-2: Gratuity Road				Inflow=1.91 cfs utflow=1.91 cfs	
Pond DE-1: Drip Edge	Discarded=0.0	Peak Elev=1.39 07 cfs 1,036 cf Prim	' Storage=0.005 af ary=0.00 cfs  0 cf   O		
Pond DE-10: Drip Edge	Discarded=0.0	Peak Elev=1.50 07 cfs 1,077 cf Prim	' Storage=0.005 af ary=0.00 cfs  0 cf   O		
Pond DE-11: Drip Edge	Discarded=0.0	Peak Elev=1.72 07 cfs 1,073 cf Prim	' Storage=0.006 af ary=0.00 cfs 0 cf O		
Pond DE-12: Drip Edge	Discarded=0.0	Peak Elev=1.50 07 cfs 1,077 cf Prim	' Storage=0.005 af ary=0.00 cfs  0 cf   O		
Pond DE-2: Drip Edge	Discarded=0.0	Peak Elev=2.00 06 cfs 1,072 cf Prim	' Storage=0.006 af ary=0.01 cfs 1 cf O		
Pond DE-3: Drip Edge	Discarded=0.0	Peak Elev=1.87 07 cfs 1,077 cf Prim	' Storage=0.006 af ary=0.00 cfs 0 cf O		
Pond DE-4: Drip Edge	Discarded=0.06	Peak Elev=2.00 5 cfs 1,008 cf Prima	' Storage=0.006 af ry=0.07 cfs 28 cf O		
Pond DE-5: Drip Edge	Discarded=0.0	Peak Elev=1.72 07 cfs 1,073 cf Prim	' Storage=0.006 af ary=0.00 cfs 0 cf O		

<b>22-243 Post Developme</b> Prepared by Landtech Cou <u>HydroCAD® 10.00-26 s/n 007</u>	nsultants, Inc.	D Software Solutio		100 Year Rainfall=7.80" Printed 1/14/2025 Page 88
Pond DE-6: Drip Edge	Discarded=0.07 cf			f Inflow=0.33 cfs 1,078 cf Outflow=0.07 cfs 1,077 cf
Pond DE-7: Drip Edge	Discarded=0.06 cf			f Inflow=0.32 cfs 1,036 cf Outflow=0.06 cfs 1,036 cf
Pond DE-8: Drip Edge	Discarded=0.07 cf			f Inflow=0.32 cfs 1,036 cf Outflow=0.07 cfs 1,036 cf
Pond DE-9: Drip Edge	Discarded=0.06 cfs			f Inflow=0.33 cfs 1,074 cf Outflow=0.16 cfs 1,073 cf
Pond GD-1: Ground Depres	sion	Peak Elev=211.80		Inflow=1.77 cfs 11,031 cf Outflow=1.39 cfs 11,028 cf
Pond GD-2: Ground Depres	sion F	Peak Elev=211.20	Storage=1,516 c	f Inflow=1.63 cfs 9,814 cf Outflow=0.98 cfs 9,809 cf
Pond GD-3: Ground Depres	sion	Peak Elev=214.3	34' Storage=607 c	f Inflow=0.89 cfs 2,645 cf Outflow=0.33 cfs 2,644 cf
Pond GD-4: Ground Depres	sion	Peak Elev=213.9	03' Storage=276 c	f Inflow=0.52 cfs 1,812 cf Outflow=0.22 cfs 1,811 cf
Pond IB-1: Infiltration Basir				Inflow=4.73 cfs 13,917 cf Dutflow=1.00 cfs 13,909 cf
Pond IB-2: Infiltration Basir				Inflow=15.41 cfs 45,516 cf Dutflow=1.92 cfs 45,480 cf
Pond IB-3: Infiltration Basir				Inflow=6.50 cfs 19,060 cf Dutflow=1.41 cfs 19,050 cf
Pond IB-4: Infiltration Basir				Inflow=11.60 cfs 34,429 cf Dutflow=1.74 cfs 34,406 cf
Total Runoff Area		noff Volume = 2 Pervious = 2,128		age Runoff Depth = 1.15" Impervious = 135,921 sf

### Summary for Subcatchment PR-1A: South of stream

Runoff = 1.71 cfs @ 14.13 hrs, Volume= 29,893 cf, Depth> 0.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

_	A	rea (sf)	CN I	Description							
-	8	97,676	30 \	Woods, Good, HSG A							
		47,438	68 ·	<50% Gras	50% Grass cover, Poor, HSG A						
		1,947	98 I	Paved park	ing, HSG A	۱					
	9	47,061	32 \	32 Weighted Average							
	9	45,114	9	99.79% Pei	rvious Area						
		1,947	(	0.21% Impe	ervious Are	а					
	То	Longth	Slope	Volocity	Conocity	Description					
	Tc (min)	Length (feet)	Slope (ft/ft)	•	Capacity (cfs)	Description					
-	31.5	50	0.0020	0.03	· · ·	Sheet Flow, A-B					
						Woods: Light underbrush n= 0.400 P2= 3.10"					
	73.4	985	0.0020	0.22		Shallow Concentrated Flow, B-C					
_						Woodland Kv= 5.0 fps					
	101 0	1 005	Tatal								

104.9 1,035 Total

#### Summary for Subcatchment PR-1B: North of stream

Runoff = 2.42 cfs @ 12.93 hrs, Volume= 29,303 cf, Depth> 0.47"

	Area (sf)	CN E	escription					
	646,242	30 V	Woods, Good, HSG A					
	67,899	68 <	<50% Grass cover, Poor, HSG A					
	33,279	30 N	Meadow, non-grazed, HSG A					
	747,420	33 Weighted Average						
	747,420	1	100.00% Pervious Area					
To	5	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	50	0.0200	0.15		Sheet Flow, A-B			
					Grass: Short n= 0.150 P2= 3.10"			
38.2	810	0.0050	0.35		Shallow Concentrated Flow, B-C			
					Woodland Kv= 5.0 fps			
43.9	860	Total						

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#### Summary for Subcatchment PR-1C: Jenkins - entrance

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[49] Hint: Tc<2dt may require smaller dt

= 4.73 cfs @ 12.07 hrs, Volume= 13,917 cf, Depth> 5.12" Runoff

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

Ar	ea (sf)	CN	Description				
	9,869	98	Paved road	s w/curbs &	& sewers, HSG A		
	19,113	68	<50% Gras	s cover, Po	oor, HSG A		
	3,635	98	Roofs, HSC	θA			
	32,617	80	Weighted Average				
	19,113		58.60% Pervious Area				
	13,504	504 41.40% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		
5.0					Direct Entry,		

#### Summary for Subcatchment PR-1D: Jenkins - Middle

[49] Hint: Tc<2dt may require smaller dt

Runoff 15.41 cfs @ 12.07 hrs, Volume= 45,516 cf, Depth> 5.23" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

Area (sf)	CN	Description							
34,458	98	Paved road	ls w/curbs &	& sewers, HSG A					
60,702	68	<50% Gras	s cover, Po	bor, HSG A					
9,198	98	Roofs, HSC	ĞΑ						
104,358	81	Weighted A	Weighted Average						
60,702		58.17% Pe	rvious Area	a					
43,656		41.83% Im	41.83% Impervious Area						
Tc Length	Slop	be Velocity	Capacity	Description					
(min) (feet)	(ft/	ft) (ft/sec)	) (ft/sec) (cfs)						
5.0				Direct Entry,					

#### Summary for Subcatchment PR-1E: Gratuity Rd - middle near entrance

[49] Hint: Tc<2dt may require smaller dt

6.50 cfs @ 12.07 hrs, Volume= 19,060 cf, Depth> 5.01" Runoff =

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A	rea (sf)	CN	Description	escription						
	15,364	98	Paved road	s w/curbs &	& sewers, HSG A					
	28,536	68	<50% Gras	s cover, Po	oor, HSG A					
	1,785	98	Roofs, HSC	βA						
	45,685	79	Weighted Average							
	28,536		62.46% Pervious Area							
	17,149		37.54% Imp	pervious Ar	rea					
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	1					
5.0					Direct Entry,					

# Summary for Subcatchment PR-1F: Gratuity Rd - cul-de-sac

[49] Hint: Tc<2dt may require smaller dt

Runoff = 11.60 cfs @ 12.07 hrs, Volume= 34,429 cf, Depth> 5.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

A	rea (sf)	CN	Description	escription						
	27,603	98	Paved road	s w/curbs &	& sewers, HSG A					
	42,235	68	<50% Gras	s cover, Po	bor, HSG A					
	7,414	98	Roofs, HSC	θA						
	77,252	82	Weighted A	Veighted Average						
	42,235		54.67% Pei	54.67% Pervious Area						
	35,017		45.33% Imp	pervious Are	ea					
т.	1			0						
Tc	Length	Slope		Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec) (cfs)							
5.0					Direct Entry,					

#### Summary for Subcatchment PR-1G: (new Subcat)

Runoff = 1.77 cfs @ 12.47 hrs, Volume= 11,031 cf, Depth> 1.01"

 Area (sf)	CN	Description					
33,650	68	<50% Grass cover, Poor, HSG A					
 97,543	30	Voods, Good, HSG A					
131,193	40	Weighted Average					
131,193		100.00% Pervious Area					

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
16.5	50	0.0100	0.05		Sheet Flow,					
8.3	250	0.0100	0.50		Woods: Ligh <b>Shallow Co</b> r Woodland	t underbrush n= 0.400 <b>ncentrated Flow,</b> Kv= 5.0 fps	P2= 3.10'	,		
24.8	300	Total								
Summary for Subcatchment PR-1H: (new Subcat)										
Runoff	=	1.63 cf	s@ 12.4	6 hrs, Volu	me=	9,814 cf, Depth> 1.09'	•			
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"									
A	rea (sf)	CN E	escription							
	29,892			s cover, Po	or, HSG A					
	77,876			od, HSG A						
	07,768 07,768		Veighted A 00.00% Pe	verage ervious Are	а					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
16.5	50	0.0100	0.05		Sheet Flow,					
8.3	250	0.0100	0.50			It underbrush n= 0.400 ncentrated Flow, Kv= 5.0 fps	P2= 3.10 <sup>°</sup>			
24.8	300	Total								
			Sum	mary for	Subcatchm	nent PR-1I:				
[49] Hint	: Tc<2dt	may requ	ire smaller	dt						
Runoff	=	0.89 cf	s@ 12.0	9 hrs, Volu	me=	2,645 cf, Depth> 2.32'	,			
			nod, UH=S ainfall=7.80		ted-CN, Time	Span= 5.00-20.00 hrs, c	lt= 0.05 hrs	3		
А	rea (sf)	CN D	escription							
	8,676			s cover, Po	or, HSG A					
	5,002			od, HSG A						
	13,678 13,678		Veighted A 00.00% Pe	verage ervious Are	а					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry	,				

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#### Summary for Subcatchment PR-1K:

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.52 cfs @ 12.10 hrs, Volume= 1,812 cf, Depth> 1.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

Ar	ea (sf)	CN	Description						
	5,678	68	<50% Gras	s cover, Po	or, HSG A				
	11,311	30	Meadow, no	on-grazed, l	HSG A				
·	16,989	43	Weighted A	Veighted Average					
	16,989		100.00% Pe	ervious Area	а				
Тс	Length	Slope		Capacity	Description				
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)					
5.0					Direct Entry,				
					-				

#### Summary for Subcatchment PR-2: Gratuity Rd - entrance

Runoff =	: 1.91 cfs @	12.14 hrs,	Volume=	6,365 cf,	Depth> 4.22"
----------	--------------	------------	---------	-----------	--------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

	A	rea (sf)	CN E	Description		
		2,680	98 F	Paved road	s w/curbs &	& sewers, HSG A
		15,434	68 <	<50% Gras	s cover, Po	bor, HSG A
-		18,114	72 V	Veighted A	verage	
		15,434	8	35.20% Per	vious Area	
		2,680	1	4.80% Imp	pervious Ar	ea
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.5	50	0.0100	0.11		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.10"
	2.3	98	0.0100	0.70		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	0 0	110	Total			

9.8 148 Total

#### Summary for Subcatchment Unit 1: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.32 cfs @ 12.07 hrs, Volume= 1,036 cf, Depth> 6.96"

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Δ	rea (sf)	CN	Description	2						
	1,785	98	Roofs, HSG	β A						
	1,785		100.00% Im		rea					
Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description					
5.0					Direct Entry	/,				
Summary for Subcatchment Unit 10: Unit C - Hickory										
[49] Hint: Tc<2dt may require smaller dt										
Runoff	=	0.33	cfs @ 12.07	7 hrs, Volu	ime=	1,078 cf, Depth> 6.96"				
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr  100 Year Rainfall=7.80"										
A	rea (sf)	CN	Description							
	1,857	98	Roofs, HSG							
	1,857		100.00% Im	pervious A	rea					
Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description					
5.0				. ,	Direct Entry	/,				
		Sur	nmary for	Subcatcl	nment Unit	11: Unit B - Hemlock				
[49] Hint	: Tc<2dt r	may ree	quire smaller	dt						
Runoff	=	0.33	cfs @ 12.07	7 hrs, Volu	ime=	1,074 cf, Depth> 6.96"				
			ethod, UH=S Rainfall=7.80		nted-CN, Time	e Span= 5.00-20.00 hrs, dt= 0.05 hrs				
Α	rea (sf)	CN	Description							
	1,850	98	Roofs, HSG							
	1,850		100.00% Im	pervious A	rea					
Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description					
5.0					Direct Entry	/,				
		Su	mmary for	Subcatc	hment Unit	12: Unit C - Hickory				
[49] Hint	: Tc<2dt r	may ree	quire smaller	dt						

Runoff 0.33 cfs @ 12.07 hrs, Volume= 1,078 cf, Depth> 6.96" =

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A	rea (sf)	CN	Description					
	1,857	98	Roofs, HSG	βA				
	1,857		100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)				
5.0					Direct Entry,			

### Summary for Subcatchment Unit 2: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.33 cfs @ 12.07 hrs, Volume= 1,074 cf, Depth> 6.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

A	rea (sf)	CN I	Description		
	1,850	98 I	Roofs, HSG	θA	
	1,850		100.00% In	npervious A	rea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

# Summary for Subcatchment Unit 3: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.33 cfs @ 12.07 hrs, Volume= 1,078 cf, Depth> 6.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

Ar	rea (sf)	CN I	Description							
	1,857	98 I	98 Roofs, HSG A							
	1,857		100.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry,					

# Summary for Subcatchment Unit 4: Unit A - Birch

1,036 cf, Depth> 6.96"

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.32 cfs @ 12.07 hrs, Volume=

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

Area (sf)	CN Descriptior	ı	
1,785	98 Roofs, HS	GΑ	
1,785	100.00% Ir	npervious A	rea
Tc Lengtł (min) (feet		Capacity (cfs)	Description
5.0			Direct Entry,

### Summary for Subcatchment Unit 5: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.33 cfs @ 12.07 hrs, Volume= 1,074 cf, Depth> 6.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

A	rea (sf)	CN I	Description		
	1,850	98 I	Roofs, HSG	βA	
	1,850		100.00% In	npervious A	vrea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

## Summary for Subcatchment Unit 6: Unit C - Hickory

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.33 cfs @ 12.07 hrs, Volume= 1,078 cf, Depth> 6.96"

A	rea (sf)	CN I	Description		
	1,857	98 I	Roofs, HSC	βA	
	1,857		100.00% In	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	
5.0					Direct Entry,

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## Summary for Subcatchment Unit 7: Unit A - Birch

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.32 cfs @ 12.07 hrs, Volume= 1,036 cf, Depth> 6.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

Ai	rea (sf)	CN	Description				
	1,785	98	Roofs, HSG A				
	1,785		100.00% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description		
5.0					Direct Entry,		
		S	ummary f	or Subca	tchment Unit 8: Unit A - Birc	h	

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.32 cfs @ 12.07 hrs, Volume= 1,036 cf, Depth> 6.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 Year Rainfall=7.80"

Ai	rea (sf)	CN [	Description		
	1,785	98 F	Roofs, HSC	βA	
	1,785		00.00% In	npervious A	vrea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

#### Summary for Subcatchment Unit 9: Unit B - Hemlock

[49] Hint: Tc<2dt may require smaller dt

Runoff = 0.33 cfs @ 12.07 hrs, Volume= 1,074 cf, Depth> 6.96"

 Area (sf)	CN	Description
1,850	98	Roofs, HSG A
 1,850		100.00% Impervious Area

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Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.0					Direct Entry,

### Summary for Reach DP-1: Stream

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	1,694,481 sf,	0.11% Impervious,	Inflow Depth > 0.42	for 100 Year event
Inflow	=	3.22 cfs @ 1	13.91 hrs, Volume=	59,196 cf	
Outflow	=	3.22 cfs @ 1	13.91 hrs, Volume=	59,196 cf, At	ten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

### Summary for Reach DP-2: Gratuity Road

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	18,114 sf, 14.80% Impervious, I	Inflow Depth > 4.22"	for 100 Year event
Inflow	=	1.91 cfs @ 12.14 hrs, Volume=	6,365 cf	
Outflow	=	1.91 cfs @ 12.14 hrs, Volume=	6,365 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

### Summary for Pond DE-1: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area =	1,785 sf,100.00% Impervious,	Inflow Depth > 6.96" for 100 Year event
Inflow =	0.32 cfs @ 12.07 hrs, Volume=	1,036 cf
Outflow =	0.07 cfs @ 11.75 hrs, Volume=	1,036 cf, Atten= 76%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.75 hrs, Volume=	1,036 cf
Primary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.39' @ 12.45 hrs Surf.Area= 0.009 ac Storage= 0.005 af

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 14.2 min (746.6 - 732.4)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	0.007 af	<b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids
Device	Routing	Invert O	utlet Devices
#1	Discarded	0.00' <b>8</b> .	270 in/hr Exfiltration over Surface area
#2	Primary	2.00' 72	2.0' long x 2.0' breadth Broad-Crested Rectangular Weir

2.50 3.00 3.50

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00

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Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.07 cfs @ 11.75 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Summary for Pond DE-10: Drip Edge

[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1) 1,857 sf,100.00% Impervious, Inflow Depth > 6.96" for 100 Year event Inflow Area = Inflow 0.33 cfs @ 12.07 hrs, Volume= 1.078 cf = Outflow 0.07 cfs @ 11.75 hrs, Volume= 1,077 cf. Atten= 77%, Lag= 0.0 min = 0.07 cfs @ 11.75 hrs, Volume= Discarded = 1,077 cf 5.00 hrs, Volume= Primary = 0.00 cfs @ 0 cf Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.50' @ 12.46 hrs Surf.Area= 0.009 ac Storage= 0.005 af Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 15.5 min (747.9 - 732.4) Volume Avail.Storage Storage Description Invert 3.00'W x 130.00'L x 2.00'H Prismatoid #1 0.00' 0.007 af 0.018 af Overall x 40.0% Voids Device Routing Invert Outlet Devices Discarded 8.270 in/hr Exfiltration over Surface area #1 0.00' 72.0' long x 2.0' breadth Broad-Crested Rectangular Weir #2 Primary 2.00' Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32 Discarded OutFlow Max=0.07 cfs @ 11.75 hrs HW=0.03' (Free Discharge)

**1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs) **22-243 Post Development - Rev 2** *Type III* Prepared by Landtech Consultants, Inc. HydroCAD® 10.00-26 s/n 00798 © 2020 HydroCAD Software Solutions LLC

### Summary for Pond DE-11: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area =	1,850 sf,100.00% Impervious,	Inflow Depth > 6.96" for 100 Year event
Inflow =	0.33 cfs @ 12.07 hrs, Volume=	1,074 cf
Outflow =	0.07 cfs @ 11.70 hrs, Volume=	1,073 cf, Atten= 79%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.70 hrs, Volume=	1,073 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.72' @ 12.48 hrs Surf.Area= 0.008 ac Storage= 0.006 af

Plug-Flow detention time= 18.5 min calculated for 1,070 cf (100% of inflow) Center-of-Mass det. time= 18.3 min (750.7 - 732.4)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	0.007 af	<b>3.00'W x 120.00'L x 2.00'H Prismatoid</b> 0.017 af Overall x 40.0% Voids
Device	Routing	Invert O	utlet Devices
#1	Discarded	0.00' <b>8.</b>	270 in/hr Exfiltration over Surface area
#2	Primary		2.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
		2.	50 3.00 3.50
		C	oef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
		2.	85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.07 cfs @ 11.70 hrs HW=0.02' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Summary for Pond DE-12: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area =	1,857 sf,100.00% Impervious,	Inflow Depth > 6.96" for 100 Year event
Inflow =	0.33 cfs @ 12.07 hrs, Volume=	1,078 cf
Outflow =	0.07 cfs @ 11.75 hrs, Volume=	1,077 cf, Atten= 77%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.75 hrs, Volume=	1,077 cf
Primary =	0.00 cfs $\overline{@}$ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

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Peak Elev= 1.50' @ 12.46 hrs Surf.Area= 0.009 ac Storage= 0.005 af
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 15.5 min ( 747.9 - 732.4 )
Volume Invert Avail.Storage Storage Description
#1 0.00' 0.007 af <b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids
Device Routing Invert Outlet Devices
#1       Discarded       0.00'       8.270 in/hr Exfiltration over Surface area         #2       Primary       2.00'       72.0' long x 2.0' breadth Broad-Crested Rectangular Weir         Head (feet)       0.20       0.40       0.60       0.80       1.00       1.20       1.40       1.60       1.80       2.00'         2.50       3.00       3.50       Coef. (English)       2.54       2.61       2.61       2.60       2.70       2.77       2.89       2.88         2.85       3.07       3.20       3.32
<b>Discarded OutFlow</b> Max=0.07 cfs @ 11.75 hrs HW=0.03' (Free Discharge) <b>1=Exfiltration</b> (Exfiltration Controls 0.07 cfs)
Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
Summary for Pond DE-2: Drip Edge
[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)
Inflow Area =       1,850 sf,100.00% Impervious, Inflow Depth > 6.96" for 100 Year event         Inflow =       0.33 cfs @ 12.07 hrs, Volume=       1,074 cf         Outflow =       0.07 cfs @ 12.50 hrs, Volume=       1,073 cf, Atten= 79%, Lag= 25.8 min         Discarded =       0.06 cfs @ 11.70 hrs, Volume=       1,072 cf         Primary =       0.01 cfs @ 12.50 hrs, Volume=       1 cf
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.00' @ 12.49 hrs Surf.Area= 0.008 ac Storage= 0.006 af
Plug-Flow detention time= 22.1 min calculated for 1,073 cf (100% of inflow) Center-of-Mass det. time= 22.0 min(754.4-732.4)
Volume Invert Avail.Storage Storage Description
#1 0.00' 0.006 af <b>3.00'W x 110.00'L x 2.00'H Prismatoid</b> 0.015 af Overall x 40.0% Voids
Device Routing Invert Outlet Devices
#1       Discarded         #2       Primary         2.00'       8.270 in/hr Exfiltration over Surface area         72.0' long x 2.0' breadth Broad-Crested Rectangular Weir         Head (feet)       0.20         0.00'       2.50         0.00'       2.50         0.00'       2.50         0.00'       2.50         0.00'       2.50         0.00'       2.50         0.00'       2.50         0.00'       2.50         0.00'       2.50         0.00'       2.50         0.00'       2.50         0.20       0.40       0.60       0.80       1.00       1.40       1.60       1.80       2.00         2.50       3.00       3.50       3.00       2.61       2.61       2.66       2.70       2.77       2.89       2.88

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2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.06 cfs @ 11.70 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 12.50 hrs HW=2.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Weir Controls 0.00 cfs @ 0.04 fps)

### Summary for Pond DE-3: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area =	1,857 sf,100.00% Impervious,	Inflow Depth > 6.96" for 100 Year event
Inflow =	0.33 cfs @ 12.07 hrs, Volume=	1,078 cf
Outflow =	0.07 cfs @ 11.70 hrs, Volume=	1,077 cf, Atten= 80%, Lag= 0.0 min
Discarded =	0.07 cfs @ 11.70 hrs, Volume=	1,077 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.87' @ 12.49 hrs Surf.Area= 0.008 ac Storage= 0.006 af

Plug-Flow detention time= 20.5 min calculated for 1,074 cf (100% of inflow) Center-of-Mass det. time= 20.2 min (752.7 - 732.4)

Volume	Invert	Avail.Storage	e Storage Description
#1	0.00'	0.006 a	af <b>3.00'W x 115.00'L x 2.00'H Prismatoid</b> 0.016 af Overall x 40.0% Voids
Device	Routing	Invert (	Dutlet Devices
#1 #2	Discarded Primary	2.00' <b>7</b> H	<b>3.270 in/hr Exfiltration over Surface area</b> <b>72.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
		C	2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.07 cfs @ 11.70 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Summary for Pond DE-4: Drip Edge

- [82] Warning: Early inflow requires earlier time span
- [92] Warning: Device #2 is above defined storage

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Inflow Area =       1,785 sf,100.00% Impervious, Inflow Depth > 6.96" for 100 Year event         Inflow =       0.32 cfs @ 12.07 hrs, Volume=       1,036 cf         Outflow =       0.13 cfs @ 12.34 hrs, Volume=       1,036 cf, Atten= 60%, Lag= 16.3 min         Discarded =       0.06 cfs @ 11.70 hrs, Volume=       1,008 cf         Primary =       0.07 cfs @ 12.34 hrs, Volume=       28 cf			
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.00' @ 12.35 hrs Surf.Area= 0.007 ac Storage= 0.006 af			
Plug-Flow detention time= 21.7 min calculated for 1,036 cf (100% of inflow) Center-of-Mass det. time= 21.6 min ( 754.0 - 732.4 )			
Volume Invert Avail.Storage Storage Description			
#1 0.00' 0.006 af <b>3.00'W x 100.00'L x 2.00'H Prismatoid</b> 0.014 af Overall x 40.0% Voids			
Device Routing Invert Outlet Devices			
#1       Discarded       0.00'       8.270 in/hr Exfiltration over Surface area         #2       Primary       2.00'       72.0' long x 2.0' breadth Broad-Crested Rectangular Weir         Head (feet)       0.20       0.40       0.60       0.80       1.00       1.20       1.40       1.60       1.80       2.00'         2.50       3.00       3.50       Coef. (English)       2.54       2.61       2.61       2.60       2.70       2.77       2.89       2.88         2.85       3.07       3.20       3.32			
Discarded OutFlow Max=0.06 cfs @ 11.70 hrs HW=0.03' (Free Discharge)			
Primary OutFlow Max=0.02 cfs @ 12.34 hrs HW=2.00' (Free Discharge) <b>1 2=Broad-Crested Rectangular Weir</b> (Weir Controls 0.02 cfs @ 0.13 fps)			
Summary for Pond DE-5: Drip Edge			
[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)			
Inflow Area =       1,850 sf,100.00% Impervious, Inflow Depth > 6.96" for 100 Year event         Inflow =       0.33 cfs @ 12.07 hrs, Volume=       1,074 cf         Outflow =       0.07 cfs @ 11.70 hrs, Volume=       1,073 cf, Atten= 79%, Lag= 0.0 min         Discarded =       0.07 cfs @ 11.70 hrs, Volume=       1,073 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf			
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.72' @ 12.48 hrs Surf.Area= 0.008 ac Storage= 0.006 af			
Plug-Flow detention time= 18.5 min calculated for 1,070 cf (100% of inflow) Center-of-Mass det. time= 18.3 min ( 750.7 - 732.4 )			

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 Type III 24-hr
 100 Year Rainfall=7.80"

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Volume	Invert	Avail.Storage	Storage Description				
#1	0.00'	0.007 af					
			0.017 af Overall x 40.0% Voids				
Device	Routing		utlet Devices				
#1 #2	Discarded Primary	2.00' <b>72</b> He 2.9 Co	8.270 in/hr Exfiltration over Surface area 72.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32				
		Max=0.07 cfs @ diltration Control	9 11.70 hrs HW=0.02' (Free Discharge) s 0.07 cfs)				
			.00 hrs HW=0.00' (Free Discharge) eir (Controls 0.00 cfs)				
		Sum	mary for Pond DE-6: Drip Edge				
[92] Wa	[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)						
Inflow A Inflow Outflow Discarde Primary	= 0. = 0. ed = 0.	.33 cfs @ 12.07	5 hrs, Volume=         1,077 cf, Atten= 77%, Lag= 0.0 min           5 hrs, Volume=         1,077 cf				
			an= 5.00-20.00 hrs, dt= 0.05 hrs Area= 0.009 ac Storage= 0.005 af				
	Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 15.5 min ( 747.9 - 732.4 )						
Volume	Invert	Avail.Storage					
#1	0.00'	0.007 af	<b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids				
Device	Routing	Invert Ou	utlet Devices				
#1 #2	Discarded Primary	2.00' <b>72</b> He 2.9 Co	<b>270 in/hr Exfiltration over Surface area</b> <b>2.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 50 3.00 3.50 bef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 85 3.07 3.20 3.32				

**Discarded OutFlow** Max=0.07 cfs @ 11.75 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Summary for Pond DE-7: Drip Edge

[92] Warning: Device #2 is above defined storage

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area =	1,785 sf,100.00% Impervious,	Inflow Depth > 6.96" for 100 Year event
Inflow =	0.32 cfs @ 12.07 hrs, Volume=	1,036 cf
Outflow =	0.06 cfs @ 11.70 hrs, Volume=	1,036 cf, Atten= 80%, Lag= 0.0 min
Discarded =	0.06 cfs @ 11.70 hrs, Volume=	1,036 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.88' @ 12.49 hrs Surf.Area= 0.008 ac Storage= 0.006 af

Plug-Flow detention time= 20.7 min calculated for 1,032 cf (100% of inflow) Center-of-Mass det. time= 20.5 min (752.9 - 732.4)

Volume	Invert	Avail.Storage	e Storage Description
#1	0.00'	0.006 a	af <b>3.00'W x 110.00'L x 2.00'H Prismatoid</b> 0.015 af Overall x 40.0% Voids
Device	Routing	Invert (	Outlet Devices
#1	Discarded	0.00' 8	3.270 in/hr Exfiltration over Surface area
#2	Primary	  2 	<b>72.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

**Discarded OutFlow** Max=0.06 cfs @ 11.70 hrs HW=0.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

### Summary for Pond DE-8: Drip Edge

[82] Warning: Early inflow requires earlier time span

[92] Warning: Device #2 is above defined storage

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

22-243 Post Development - Rev 2Type III 24-hr100 Year Rainfall=7.80"Prepared by Landtech Consultants, Inc.Printed1/14/2025HydroCAD® 10.00-26 s/n 00798 © 2020 HydroCAD Software Solutions LLCPage 106					
Inflow Area =       1,785 sf,100.00% Impervious, Inflow Depth > 6.96" for 100 Year event         Inflow =       0.32 cfs @ 12.07 hrs, Volume=       1,036 cf         Outflow =       0.07 cfs @ 11.75 hrs, Volume=       1,036 cf, Atten= 76%, Lag= 0.0 min         Discarded =       0.07 cfs @ 11.75 hrs, Volume=       1,036 cf         Primary =       0.00 cfs @ 5.00 hrs, Volume=       0 cf					
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1.39' @ 12.45 hrs Surf.Area= 0.009 ac Storage= 0.005 af					
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 14.2 min ( 746.6 - 732.4 )					
Volume Invert Avail.Storage Storage Description					
#1 0.00' 0.007 af <b>3.00'W x 130.00'L x 2.00'H Prismatoid</b> 0.018 af Overall x 40.0% Voids					
Device Routing Invert Outlet Devices					
#1       Discarded         #2       Primary         2.00'       8.270 in/hr Exfiltration over Surface area         72.0' long x 2.0' breadth Broad-Crested Rectangular Weir         Head (feet)       0.20         0.50       3.00         2.00'       72.0' long x 2.0' breadth Broad-Crested Rectangular Weir         Head (feet)       0.20         0.50       3.00         2.50       3.00         2.50       3.00         2.50       3.00         2.54       2.61         2.60       2.70         2.77       2.89         2.85       3.07         3.20       3.32					
Discarded OutFlow Max=0.07 cfs @ 11.75 hrs HW=0.03' (Free Discharge) ☐ 1=Exfiltration (Exfiltration Controls 0.07 cfs)					
Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=0.00' (Free Discharge) <b>1 —2=Broad-Crested Rectangular Weir</b> (Controls 0.00 cfs)					
Summary for Pond DE-9: Drip Edge					
[82] Warning: Early inflow requires earlier time span [92] Warning: Device #2 is above defined storage [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)					
Inflow Area =       1,850 sf,100.00% Impervious, Inflow Depth > 6.96" for 100 Year event         Inflow =       0.33 cfs @ 12.07 hrs, Volume=       1,074 cf         Outflow =       0.16 cfs @ 12.30 hrs, Volume=       1,073 cf, Atten= 50%, Lag= 13.8 min         Discarded =       0.06 cfs @ 11.70 hrs, Volume=       1,040 cf         Primary =       0.11 cfs @ 12.30 hrs, Volume=       34 cf					
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 2.00' @ 12.30 hrs Surf.Area= 0.007 ac Storage= 0.006 af					
Plug-Flow detention time= 21.8 min calculated for 1,073 cf (100% of inflow) Center-of-Mass det. time= 21.7 min ( 754.1 - 732.4 )					

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<u>Volume</u> #1	<u>Invert</u> 0.00'	Avail.Storag 0.006 a	Storage Description          af       3.00'W x 102.00'L x 2.00'H Prismatoid         0.014 af Overall x 40.0% Voids			
Device #1 #2	Routing Discarded Primary	0.00' 2.00'	Outlet Devices           8.270 in/hr Exfiltration over Surface area           72.0' long x 2.0' breadth Broad-Crested Rectangular Weir           Head (feet)         0.20         0.40         0.60         0.80         1.00         1.20         1.40         1.60         1.80         2.00           2.50         3.00         3.50         Coef. (English)         2.54         2.61         2.60         2.66         2.70         2.77         2.89         2.88           2.85         3.07         3.20         3.32			
<b>Discarded OutFlow</b> Max=0.06 cfs @ 11.70 hrs HW=0.03' (Free Discharge)						

**1=Exfiltration** (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.05 cfs @ 12.30 hrs HW=2.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Weir Controls 0.05 cfs @ 0.16 fps)

#### Summary for Pond GD-1: Ground Depression

Inflow Area =	131,193 sf, 0.00% Impervious,	Inflow Depth > 1.01" for 100 Year event
Inflow =	1.77 cfs @ 12.47 hrs, Volume=	11,031 cf
Outflow =	1.39 cfs @ 12.70 hrs, Volume=	11,028 cf, Atten= 22%, Lag= 13.9 min
Discarded =	1.39 cfs @ 12.70 hrs, Volume=	11,028 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 211.80' @ 12.70 hrs Surf.Area= 7,237 sf Storage= 872 cf

Plug-Flow detention time= 4.2 min calculated for 10,991 cf (100% of inflow) Center-of-Mass det. time= 4.1 min (870.8 - 866.7)

Volume	Invert	Avail.Sto	orage Storag	e Description	
#1	211.60'	2,9	03 cf Custo	m Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
211.6	60	1,514	0	0	
212.0	0	13,000	2,903	2,903	
Device	Routing	Invert	Outlet Devic	ces	
#1	Discarded	211.60'	8.270 in/hr	Exfiltration over	Surface area

**Discarded OutFlow** Max=1.39 cfs @ 12.70 hrs HW=211.80' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.39 cfs)

### Summary for Pond GD-2: Ground Depression

Inflow Area =	107,768 sf, 0.00% Impervious,	Inflow Depth > 1.09" for 100 Year event
Inflow =	1.63 cfs @ 12.46 hrs, Volume=	9,814 cf
Outflow =	0.98 cfs @ 12.82 hrs, Volume=	9,809 cf, Atten= 40%, Lag= 21.6 min
Discarded =	0.98 cfs @ 12.82 hrs, Volume=	9,809 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 211.20' @ 12.82 hrs Surf.Area= 5,125 sf Storage= 1,516 cf

Plug-Flow detention time= 15.5 min calculated for 9,809 cf (100% of inflow) Center-of-Mass det. time= 15.3 min ( 878.5 - 863.2 )

Volume	Invert	Avail.Sto	rage	Storage	Description	
#1	210.60'	3,54	47 cf	Custom	Stage Data (Pri	smatic) Listed below (Recalc)
Elevation (feet)	Sur	f.Area (sq-ft)	Inc. (cubic	Store -feet)	Cum.Store (cubic-feet)	
210.60		550		0	0	
211.00		2,998		710	710	
211.50		8,352	2	2,838	3,547	
-	outing iscarded	Invert 210.60'		t Device	s Afiltration over S	Surface area
"T D		210.00	0.270			

**Discarded OutFlow** Max=0.98 cfs @ 12.82 hrs HW=211.20' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.98 cfs)

#### Summary for Pond GD-3: Ground Depression

Inflow Area =	13,678 sf, 0.00% Impervious,	Inflow Depth > 2.32"	for 100 Year event
Inflow =	0.89 cfs @ 12.09 hrs, Volume=	2,645 cf	
Outflow =	0.33 cfs @ 12.41 hrs, Volume=	2,644 cf, Atter	n= 63%, Lag= 19.4 min
Discarded =	0.33 cfs @ 12.41 hrs, Volume=	2,644 cf	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 214.34' @ 12.41 hrs Surf.Area= 1,712 sf Storage= 607 cf

Plug-Flow detention time= 18.5 min calculated for 2,636 cf (100% of inflow) Center-of-Mass det. time= 18.3 min (835.6 - 817.3)

Volume	Invert	Avail.	Storage	Storage	e Description	
#1	213.60'		919 cf	Custon	n Stage Data (Pri	ismatic) Listed below (Recalc)
Elevation (feet)	Surf./ (s	Area sq-ft)		.Store c-feet)	Cum.Store (cubic-feet)	
213.60		355		0	0	
214.00		660		203	203	
214.50	2	,205		716	919	

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Device	Routing	Invert	Outlet Devices
#1	Discarded	213.60'	8.270 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=0.33 cfs @ 12.41 hrs HW=214.34' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.33 cfs)

#### Summary for Pond GD-4: Ground Depression

Inflow Area =	16,989 sf, 0.00% Impervious,	Inflow Depth > 1.28" for 100 Year event
Inflow =	0.52 cfs @ 12.10 hrs, Volume=	1,812 cf
Outflow =	0.22 cfs @ 12.45 hrs, Volume=	1,811 cf, Atten= 58%, Lag= 20.8 min
Discarded =	0.22 cfs @ 12.45 hrs, Volume=	1,811 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.93' @ 12.45 hrs Surf.Area= 1,133 sf Storage= 276 cf

Plug-Flow detention time= 7.8 min calculated for 1,811 cf (100% of inflow) Center-of-Mass det. time= 7.7 min (850.4 - 842.7)

Volume	Invert	Avail.Sto	rage Storag	ge Description	
#1	213.60'	30	63 cf Custo	om Stage Data (Pris	matic) Listed below (Recalc)
Elevatio (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
213.6	-	550	0	0	
214.0	0	1,263	363	363	
Device	Routing	Invert	Outlet Devi	ces	
#1	Discarded	213.60'	8.270 in/hr	Exfiltration over Su	Irface area

**Discarded OutFlow** Max=0.22 cfs @ 12.45 hrs HW=213.93' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.22 cfs)

### Summary for Pond IB-1: Infiltration Basin #1

Inflow Area =	32,617 sf, 41.40% Impervious,	Inflow Depth > 5.12" for 100 Year event
Inflow =	4.73 cfs @ 12.07 hrs, Volume=	13,917 cf
Outflow =	1.00 cfs @ 12.50 hrs, Volume=	13,909 cf, Atten= 79%, Lag= 25.3 min
Discarded =	1.00 cfs @ 12.50 hrs, Volume=	13,909 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 211.77' @ 12.50 hrs Surf.Area= 5,220 sf Storage= 3,754 cf

Plug-Flow detention time= 23.9 min calculated for 13,909 cf (100% of inflow) Center-of-Mass det. time= 23.7 min (794.9 - 771.2)

Volume	Invert	Avail.Storage	Storage Description
#1	211.00'	11,236 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
211.00	4,480	0	0
212.00	5,436	4,958	4,958
213.00	7,119	6,278	11,236

Device	Routing	Invert	Outlet Devices
#1	Primary	212.00'	5.0' long x 15.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Discarded	211.00'	8.270 in/hr Exfiltration over Surface area

**Discarded OutFlow** Max=1.00 cfs @ 12.50 hrs HW=211.77' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 1.00 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=211.00' (Free Discharge) ☐ 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Summary for Pond IB-2: Infiltration Basin #2

Inflow Area =	104,358 sf, 41.83% Impervious,	Inflow Depth > 5.23" for 100 Year event
Inflow =	15.41 cfs @ 12.07 hrs, Volume=	45,516 cf
Outflow =	1.92 cfs @ 12.66 hrs, Volume=	45,480 cf, Atten= 88%, Lag= 35.1 min
Discarded =	1.92 cfs @ 12.66 hrs, Volume=	45,480 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 213.85' @ 12.66 hrs Surf.Area= 10,042 sf Storage= 16,558 cf

Plug-Flow detention time= 70.3 min calculated for 45,329 cf (100% of inflow) Center-of-Mass det. time= 69.7 min (838.8 - 769.1)

Volume	Inver	t Avail.Sto	rage Storage	e Description	
#1	212.00	' 28,9	39 cf Custor	n Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
212.0	00	7,901	0	0	
213.0	00	9,033	8,467	8,467	
214.0	00	10,222	9,628	18,095	
215.0	00	11,467	10,845	28,939	
Device	Routing	Invert	Outlet Devic	es	
#1	Discarded	212.00'	8.270 in/hr E	Exfiltration over	Surface area
#2	Primary	214.00'	Head (feet)	0.20 0.40 0.60	Dad-Crested Rectangular Weir 0.80 1.00 1.20 1.40 1.60 .70 2.64 2.63 2.64 2.64 2.63

**Discarded OutFlow** Max=1.92 cfs @ 12.66 hrs HW=213.85' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.92 cfs)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=212.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

#### Summary for Pond IB-3: Infiltration Basin #3

Inflow Area =	45,685 sf, 37.54% Impervious,	Inflow Depth > 5.01" for 100	Year event
Inflow =	6.50 cfs @ 12.07 hrs, Volume=	19,060 cf	
Outflow =	1.41 cfs @ 12.49 hrs, Volume=	19,050 cf, Atten= 78%, I	_ag= 25.0 min
Discarded =	1.41 cfs @ 12.49 hrs, Volume=	19,050 cf	
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 214.22' @ 12.49 hrs Surf.Area= 7,371 sf Storage= 5,023 cf

Plug-Flow detention time= 22.4 min calculated for 18,987 cf (100% of inflow) Center-of-Mass det. time= 22.1 min (795.3 - 773.2)

Volume	Inver	t Avail.Stor	rage Storag	ge Description
#1	213.50	' 15,68	31 cf Custo	om Stage Data (Prismatic) Listed below (Recalc)
	_			
Elevatio		urf.Area	Inc.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)
213.5	50	6,490	0	0
214.0	00	7,094	3,396	3,396
214.5	50	7,710	3,701	7,097
215.0	00	8,343	4,013	11,110
215.5	50	9,940	4,571	15,681
Device	Routing	Invert	Outlet Devi	ices
#1	Discarded	213.50'	8.270 in/hr	Exfiltration over Surface area
#2	Primary	214.50'	5.0' long x	3.0' breadth Broad-Crested Rectangular Weir
	,			) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			, ,	3.50 4.00 4.50
			Coef. (Engl	lish) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68
			· · ·	2.92 2.97 3.07 3.32
			- (- (- )	

**Discarded OutFlow** Max=1.41 cfs @ 12.49 hrs HW=214.22' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 1.41 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=213.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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### Summary for Pond IB-4: Infiltration Basin #4

Inflow Area =	77,252 sf, 45.33% Impervious,	Inflow Depth > 5.35" for 100 Year event
Inflow =	11.60 cfs @ 12.07 hrs, Volume=	34,429 cf
Outflow =	1.74 cfs @ 12.57 hrs, Volume=	34,406 cf, Atten= 85%, Lag= 29.8 min
Discarded =	1.74 cfs @ 12.57 hrs, Volume=	34,406 cf
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 212.98' @ 12.57 hrs Surf.Area= 9,107 sf Storage= 11,623 cf

Plug-Flow detention time= 51.3 min calculated for 34,406 cf (100% of inflow) Center-of-Mass det. time= 51.0 min ( 818.0 - 767.0 )

Volume	Inver	t Avail.Sto	rage Stora	rage Description	
#1	211.50	' 21,63	38 cf Cus	stom Stage Data (Prismatic) Listed below (Recalc)	
Flovetic		urf Aree	Inc Stor	cum Store	
Elevatio		urf.Area	Inc.Store		
(fee	et)	(sq-ft)	(cubic-feet	t) (cubic-feet)	
211.5	50	6,864	(	0 0	
212.0	00	7,370	3,559	9 3,559	
213.0	00	9,144	8,257	7 11,816	
214.0	00	10,501	9,823	3 21,638	
Device	Routing	Invert	Outlet De	evices	
#1	Discarded	211.50'	8.270 in/h	hr Exfiltration over Surface area	
#2	Primary	213.00'	5.0' long	x 15.0' breadth Broad-Crested Rectangular Weir	
	2		Head (fee	et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60	
			Coef. (En	nglish) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63	
		Max=1.74 cf	-	hrs HW=212.98' (Free Discharge)	

**1=Exfiltration** (Exfiltration Controls 1.74 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=211.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)



**Recharge Volume Calculations (Part I and II)** 



Project: Location: Calculated By: Checked By: Date: Revised:

Gratuity Brook Farm Estates 63 Grauity Road, Groton, MA MJS MAW 7/1/2024 1/8/2025

		I	Recharge A	rea Desid	an			
Objective:	Size an infiltration bas	Size an infiltration basin that will approximate the annual recharge from the existing conditions						
Methodology:	MA Department of En	MA Department of Environmental Protection (DEP) Stormwater Management (Vol.3, Ch.1)						
Design Criteria:		times the impervious areas covering that Conservation Service (NRCS) Soil Survey						
	Hydrologic S	0	Soil Texture	Tara	et Depth Factor	(E)		
	A		Sand	0.60	inches			
	В		Loam	0.35	inches			
	С		Silty Loam	0.25	inches			
	D		Clay	0.10	inches			
		Impervious Area			Adjusted			
Recharge Volume	Total increase in	to Recharge	Adjustment	Target	Volume			
Required:	Impervious Area (sf)	BMP (sf)	Factor	Depth (in)	Required (cf)*			
	139,628	109,326	1.277	0.60	8,916			
Recharge Volume Provided:		Proposed Impervious Area (Sq. Ft.)	<u>Volume</u> <u>Required</u> (Cu. Ft.)	<u>Volume</u> Provided* (Cu. Ft.)				
	Infiltration Basin #1	13,504	675	4,958	(standard is me	et)		
		43,656						

857

1,751

7,097

11,816

(standard is met)

(standard is met)

\*-Volume provided at overflow weir elevation.

17,149

35,017

Infiltration Basin #3

Infiltration Basin #4



Project: Location: Calculated By: Checked By: Date: Revised: Gratuity Brook Farm Estates 63 Grauity Road, Groton, MA MJS MAW 7/1/2024 1/8/2025

#### RECHARGE VOLUME CALCULATIONS, PART II

**Drawdown Calculations** Objective: Size an infiltration basin that will approximate the annual recharge from the existing conditions Methodology: MA Department of Environmental Protection (DEP) Stormwater Management (Vol. 3, Ch. 1) Design Criteria: The required recharge volume equals a depth of runoff corresponding to the soil type times the impervious areas covering that soil type at the post-development site. Based on the Site Hydrologic Soil Group: Hydrologic Soil Group Target Depth Factor (F) Soil Texture Sand 0.60 inches Α В Loam 0.35 inches С 0.25 Silty Loam inches D Clay 0.10 inches Required Drawdown Time: Maximum of 72 Hours using the following equation: R<sub>v</sub> = Required Recharge Volume  $R_v$ K = the Permeability Rate Drawdown Time = (KxA<sub>Bot</sub>) A<sub>Bot</sub> = Bottom area of Infiltration basin Infiltration Basin #1 Bottom Area 4840 sf Depth 2 ft % Voids 100% 4958 cf (HydroCAD) Volume provided Rv  $\boldsymbol{A}_{\text{Bot}}$ Drawdown Time κ in/hr cf sf Hours 4,958 8.27 4,840 1.49 1.49 hours is less than 72 hours (standard is met) Infiltration Basin #2 Bottom Area 7901 sf 3 ft Depth % Voids 100% Volume provided 18095 cf (HydroCAD)  $R_v$ κ  $\boldsymbol{A}_{\text{Bot}}$ Drawdown Time in/hr Hours sf cf 18,095 8.27 7,901 3.32 3.32 hours is less than 72 hours (standard is met) Infiltration Basin #3 Bottom Area 6490 sf Depth 2 ft 100% % Voids Volume provided 7097 cf (HydroCAD)  $\boldsymbol{A}_{\text{Bot}}$ R Drawdown Time κ cf in/hr sf Hours 7,097 8.27 6,490 1.59 1.59 hours is less than 72 hours (standard is met) Infiltration Basin #4 6864 sf Bottom Area 3 ft Depth % Voids 100% Volume provided 11816 cf (HydroCAD)  $R_v$ κ  $\boldsymbol{A}_{\text{Bot}}$ Drawdown Time in/hr sf Hours cf 11,816 8.27 6,864 2.50 2.50 hours is less than 72 hours (standard is met)



Water Quality Treatment Volume Calculations



Project: Location: Calculated By: Checked By: Date: Revised: 63 Gratuity Road

Groton, MA

MJS MAW

7/1/2024

1/8/2025

#### WATER QUALITY VOLUME CALCULATIONS

 Objective:
 To determine the required Water Quality Volume (WQV) for adequte stormwater treatment

 Methodology:
 MA Department of Environmental Protection (DEP) Stormwater Management (Vol. 3, Ch. 1)

 Design Criteria:
 Volume to be treated = 1.0" x Post Development Impervious Area (Project is not classified as an area of Higher Potential Pollutant Loading)

 Volume to be
 Proposed
 Volume Required
 Water Quality

volume to be		Proposed	Volume Required	Water Quality	
Treated & Volume		Impervious Area	to be Treated	Volume Provided*	
Provided:		<u>(Sq. Ft.)</u>	<u>(Cu. Ft.)</u>	<u>(Cu. Ft.)</u>	
	Infiltration Basin #1	13,504	1,125	4,958	Standard is met
	Infiltration Basin #2	43,656	3,638	18,095	Standard is met
	Infiltration Basin #3	17,149	1,429	7,097	Standard is met
	Infiltration Basin #4	35,017	2,918	11,816	Standard is met

\* - Volume provided below basin outlet elevations.



**TSS** Calculations



#### TOTAL SUSPENDED SOLIDS (TSS) REMOVAL WORKSHEET

Legend:

= TSS Removal Rate Prior to Treatment

	Treatment Train 1:	Infiltration Basins 1, 2,				
	Α	В	С	D	E	F
		TSS Removal	Starting TSS	Amount	Remaining	TSS Removal
	BMP	Rate	Load	Removed (BxC)	Load (C-D)	Rate
PRETREATMENT	Deep Sump and Hooded Catch Basin	0.25	1.00	0.25	0.75	25%
PRETREATMENT	Stormceptor	0.75	0.75	0.56	0.19	81%
PRETREATMENT	Sediment Forebay	0.25	0.19	0.05	0.14	86%

	Landle		Project: Location: Calculated By: Checked By: Date: Revised Date:	Gratuity Brook Farm Est 63 Gratuity Road, Groto MJS MAW 8/9/2024 1/8/2025		
ΤΟΤΑ	AL SUSPENDED SOLID	OS (TSS) REMOVAL W	ORKSHEET			
Lege	nd:		= TSS Removal Rate F	Prior to Treatment		
	Treatment Train:	Pretreatment for Infiltr		_	_	_
	A	B	C C	D	E	F
	DUD	TSS Removal	Starting TSS	Amount	Remaining	TSS Removal
	BMP	Rate	Load	Removed (BxC)	Load (C-D)	Rate
PRETREATMENT	Stormceptor	0.75	1.00	0.75	0.25	75%
PRETREATMENT	Sediment Forebay	0.25	0.25	0.06	0.19	81%



**Sediment Forebay Sizing** 



Project:6Location:CCalculated By:MChecked By:MDate:7Revised:1

63 Gratuity Road Groton, MA MJS MAW 7/1/2024 1/6/2025

#### SEDIMENT FOREBAY SIZING CALCULATIONS

Objective: To determine the required Sediment Forebay volume for adequte pretreatment of water quality volume and annual sediment loading.

Methodology: MA Department of Environmental Protection (DEP) Stormwater Management (Vol. 2, Ch. 2)

 Design Criteria:
 1. Sediment Forebay Volume = 0.1" Runoff per Acre of Post Development Impervious Area

 2. Annual sediment loading (CF/year) = Area to be sanded (ac.) x 500 lb/ac.-storm / 90 lb/ft3 x 10 storm/year

#### Volume to be

Treated:

		<b>Required</b>	Required	
	Proposed	Sediment	Sediment Forebay	Provided
	Impervious Area	Forebay Volume	Volume (ASL)	Sediment Forebay
	<u>(Sq. Ft.)</u>	<u>(WQV) (Cu. Ft.)</u>	<u>(Cu. Ft.)</u>	<u>Volume (Cu. Ft.)</u>
Infiltration Basin #1	13,504	113	17	350
Infiltration Basin #2	43,656	364	56	1,338
Infiltration Basin #3	17,149	143	22	389
Infiltration Basin #4	35,017	292	45	341



**Pipe Calculations** 

#### Rainfall Data from US Department of Agriculture, Urban Hydrology for Small Watersheds - Technical Release 55

25 YEAR Design Rainfall Intensity (in/hr) =	5.5
Minimum Velocity: 25-yr Storm (fps) =	2.5
100 YEAR Design Rainfall Intensity (in/hr) =	7.8
Minimum Velocity: 100y-r Storm (fps) =	15.0

Pipe Types	Manning's Coefficient
Plastic	0.009
Steel	0.012
Concrete	0.013

PIPE NETWORK 1	LOCA	TION	Di	RAINAGE A	AREA			PIPE INFOR	MATION					PIPE FLOW					DESI	GN FLOW				FLOW PA	100 YEAR STORM				
												Actu	al Flow	Full Flow		2	25 YEAR Storm		100 YEAR Storm		torm	25 YE.	AR Storm	100 YEAR Storm			СВ	PONDING	
Description	From	То	Area Ac	Runoff Coeff., C	C*A	Invert In, Elv.	Invert Out, Elv.	Length (Feet)	Slope Ft./Ft.	Pipe Dia. (inches)	Pipe Material	Minimum (25-yr)	Maximum (100-yr)	Capacity (Cfs)	Sufficient Flow?	Velocity (fps)	Depth (Ft.)	Velocity (Fps.)	Satisfy Min. Vel.?	Depth (Ft.)	Velocity (Fps.)	Satisfy Max. Vel.?	q/Qfull 25 YEAR	Hydraulic Radius, 25 yr	q/Qfull 25 YEAR	Hydraulic Radius, 100 yr	PIPE % FULL	OPENING (SQ.IN.)	OVER CB (IN)
INDIVIDUAL SYS	TEMS (CATCH	BASINS)																											
Pipe 1	DCB-1	DMH-1	1.14	0.61	0.70	213.35	213.15	10	0.0200	15	RCP	3.9	5.5	9.1	YES	7.4	0.57	7.1	YES	0.70	7.7	YES	0.43	0.29	0.60	0.33	57	498	3.0
Pipe 2	CB-2	DMH-1	0.68	0.66	0.45	213.35	213.15	10	0.0200	15	RCP	2.5	3.5	9.1	YES	7.4	0.44	6.3	YES	0.53	6.9	YES	0.27	0.24	0.38	0.28	41	498	1.2
Pipe 3	CB-3	DMH-2	0.37	0.55	0.20	212.30	212.10	10	0.0200	12	RCP	1.1	1.6	5.0	YES	6.4	0.32	5.2	YES	0.39	5.7	YES	0.22	0.18	0.32	0.21	36	498	0.3
Pipe 4	CB-4	DMH-2	0.37	0.55	0.20	212.30	212.10	10	0.0200	12	RCP	1.1	1.6	5.0	YES	6.4	0.32	5.2	YES	0.39	5.7	YES	0.22	0.18	0.32	0.21	36	498	0.3
Pipe 5	CB-5	DMH-3	0.52	0.53	0.28	214.60	214.50	8	0.0125	12	RCP	1.5	2.2	4.0	YES	5.1	0.43	4.7	YES	0.53	5.2	YES	0.38	0.22	0.55	0.26	54	498	0.5
Pipe 6	CB-6	DMH-3	0.52	0.53	0.28	214.60	214.50	8	0.0125	12	RCP	1.5	2.2	4.0	YES	5.1	0.43	4.7	YES	0.53	5.2	YES	0.38	0.22	0.55	0.26	54	498	0.5
Pipe 7	CB-7	DMH-4	0.89	0.57	0.51	212.50	211.70	80	0.0100	15	RCP	2.8	4.0	6.5	YES	5.3	0.57	5.0	YES	0.71	5.6	YES	0.43	0.29	0.62	0.34	59	498	1.6
Pipe 8	CB-8	DMH-4	0.89	0.57	0.51	212.50	211.70	60	0.0133	12	RCP	2.8	4.0	4.1	YES	5.2	0.60	5.7	YES	0.80	5.9	YES	0.68	0.28	0.98	0.30	86	498	1.6
COMBINED SYS	TEMS											_		-					_										
Pipe 9	DMH-1	CDS			1.15	213.05	212.90	15	0.0100	18	RCP	6.3	9.0	10.5	YES	5.9	0.84	6.2	YES	1.07	6.7	YES	0.60	0.40	0.86	0.45	76		
Pipe 10	DMH-2	CDS			0.40	212.00	211.90	10	0.0100	12	RCP	2.2	3.1	3.6	YES	4.6	0.56	4.8	YES	0.71	5.1	YES	0.61	0.27	0.86	0.30	76		
Pipe 11	DMH-3	CDS			0.56	214.50	214.40	20	0.0050	15	RCP	3.1	4.4	4.6	YES	3.7	0.75	4.0	YES	0.98	4.3	YES	0.67	0.35	0.96	0.38	84		

#### Pipe Calculations



Groundwater Mounding Analysis



#### Infiltration Basin #1

Bottom of bed elevation (Bot. of Stone)	211.00 ft
Estimated seasonal high water table	208.90 ft
Clearance Provided	2.10 ft
Maximum Groundwater Mounding	0.6 ft.

#### **GROUNDWATER MOUNDING ANALYSIS USING HANTUSH EQUATION**

Input Values		inch/hour feet/day
0.5000 R	Recharge (infiltration) rate (feet/day)	0.67 1.33
0.260 Sy	Specific yield, Sy (dimensionless, between 0 and 1)	
55.00 K	Horizontal hydraulic conductivity, Kh (feet/day)*	2.00 4.00
18.000 x	1/2 length of basin (x direction, in feet)	In the report accompanying this spreadsheet (USGS SIR 2010-5102), vertical soil permeabilit
65.000 y	1/2 width of basin (y direction, in feet)	hours days (ft/d) is assumed to be one-tenth horizontal
1.000 t	duration of infiltration period (days)	36 1.50 hydraulic conductivity (ft/d).
10.000 hi(0)	initial thickness of saturated zone (feet)	
10.624 h(max)	maximum thickness of saturated zone (beneath center of	of basin at end of infiltration period)
0.624 Δh(max)	maximum groundwater mounding (beneath center of ba	asin at end of infiltration period)
Ground- Distance from		
water center of bas		
Mounding, in in x direction	in	
feet feet	<b>N A</b>	
0.624 0	Re-Calculate Now	
0.495 20		
0.301 40		
	Groundwater Mou	nding, in feet
0.173 60		8,
0.093 80	0.700	
0.067 90	0.600	
0.047 100	0.500	
0.023 120	0.400	
120		
	0.300	
	0.200	2
	0.100	
	0.000	
	0 20 40 60	80 100 120 140
Disclaimer		



#### Infiltration Basin #2

Bottom of bed elevation (Bot. of Stone)	212.00 ft
Estimated seasonal high water table	209.80 ft
Clearance Provided	2.20 ft
Maximum Groundwater Mounding	1.6 ft.

#### **GROUNDWATER MOUNDING ANALYSIS USING HANTUSH EQUATION**

	use consistent units (e.g. feet & days OT inches & hours) Conversion Table
Input Values	inch/hour feet/day
1.0500 R	Recharge (infiltration) rate (feet/day) 0.67 1.33
0.260 Sy	Specific yield, Sy (dimensionless, between 0 and 1)
55.00 K	Horizontal hydraulic conductivity, Kh (feet/day)* 2.00 4.00
25.000 x	1/2 length of basin (x direction, in feet) In the report accompanying this spreadsheet (USGS SIR 2010-5102), vertical soil permeabil
65.000 y	1/2 width of basin (y direction, in feet) hours days (fft/d) is assumed to be one-tenth horizontal
1.000 t	duration of infiltration period (days) 36 1.50 hydraulic conductivity (ft/d).
10.000 hi(0)	initial thickness of saturated zone (feet)
11.647 h(max)	maximum thickness of saturated zone (beneath center of basin at end of infiltration period)
1.647 Δh(max)	maximum groundwater mounding (beneath center of basin at end of infiltration period)
Ground- Distance from	
water center of basin	
Mounding, in in x direction, in	
feet feet	
1.647 0	Re-Calculate Now
1.419 20	Re-Calculate Now
9.897 40	
0.690 50	Communities Maximum in a in fact
0.522 60	Groundwater Mounding, in feet
0.389 70	1.800
0.285 80	1.600
0.205 90	1.400
0.146 100	1.200
9.070 120	1.000
	0.800
	0.600
	0.400
	0.200
	0.000
	0 20 40 60 80 100 120 140
Disclaimer	ivo i logados bolos reales envor despon destados constantes



#### Infiltration Basin #3

Bottom of bed elevation (Bot. of Stone)	213.50 ft
Estimated seasonal high water table	211.40 ft
Clearance Provided	2.10 ft
Maximum Groundwater Mounding	0.8 ft.

#### **GROUNDWATER MOUNDING ANALYSIS USING HANTUSH EQUATION**

Input Values	3	inch/hour feet/day
0.6000	R	Recharge (infiltration) rate (feet/day) 0.67 1.33
0.260	Sy	Specific yield, Sy (dimensionless, between 0 and 1)
55.00	К	Horizontal hydraulic conductivity, Kh (feet/day)* 2.00 4.00 In the report accompanying this spreadsheet
18.000	x	1/2 length of basin (x direction, in feet) In the report accompanying this operaciset (USGS Stra 2010-5102), vertical soil permeability.
83.000	У	1/2 width of basin (y direction, in feet) hours days (ft/d) is assumed to be one-tenth horizontal
1.000	t	duration of infiltration period (days) 36 1.50 hydraulic conductivity (ft/d).
10.000	hi(0)	initial thickness of saturated zone (feet)
10,792	h(max)	maximum thickness of saturated zone (beneath center of basin at end of infiltration period)
0.792	Δh(max)	maximum groundwater mounding (beneath center of basin at end of infiltration period)
Ground-	Distance from	
vater	center of basin	
Aounding, in	in x direction, in	
eet	feet	
0.792	0	Re-Calculate Now
0.685	20	Re-Calculate Now
0.395	40	
0.304	50	Groundwater Mounding, in feet
0.230	60	Groundwater Mounding, in feet
0.171	70	0.900
0.125	80	0.800
0.090	90	0.700
0.064	100	0.600
0.031	120	0.500
		0.400
		0.300
		0.200
		0.100
		0.000
Dicelai		0 20 40 60 80 100 120 140

Disclaimer



#### Infiltration Basin #4

Bottom of bed elevation (Bot. of Stone)	211.50 ft
Estimated seasonal high water table	208.80 ft
Clearance Provided	2.70 ft
Maximum Groundwater Mounding	1.8 ft.

#### **GROUNDWATER MOUNDING ANALYSIS USING HANTUSH EQUATION**

Input Values		inch/hour feet/day
1.0000	R	Recharge (infiltration) rate (feet/day) 0.67 1.33
0.260	Sy	Specific yield, Sy (dimensionless, between 0 and 1)
55.00	ĸ	Horizontal hydraulic conductivity, Kh (feet/day)* 2.00 4.00
34.000	x	1/2 length of basin (x direction, in feet) In the report accompanying this spreadsheet (USGS SIR 2010-5102), vertical soil permeabilit
52.000	y y	1/2 width of basin (y direction, in feet) hours days (ft/d) is assumed to be one-tenth hours to be one-tenth hours
1.000	t	duration of infiltration period (days) 36 1.50 hydraulic conductivity (fr/d).
10.000	hi(0)	initial thickness of saturated zone (feet)
11.778	h(max)	maximum thickness of saturated zone (beneath center of basin at end of infiltration period)
1.778	Δh(max)	maximum groundwater mounding (beneath center of basin at end of infiltration period)
Ground-	Distance from	
water	center of basin	
	in x direction, in	
feet	feet	
1.775	0	Pro Calendaria Narra
1.608	20	Re-Calculate Now
1.093	40	
0.840	50	Consumptions Maximalians in Sect
0.633	60	Groundwater Mounding, in feet
0.473	70	2.000
0.343	80	1.800
	90	1.600
0.179	100	1.400
	120	1.200
		0.800
		0.600
		0.400
		0.000
		0 20 40 60 80 100 120 140
Disclai	mer	

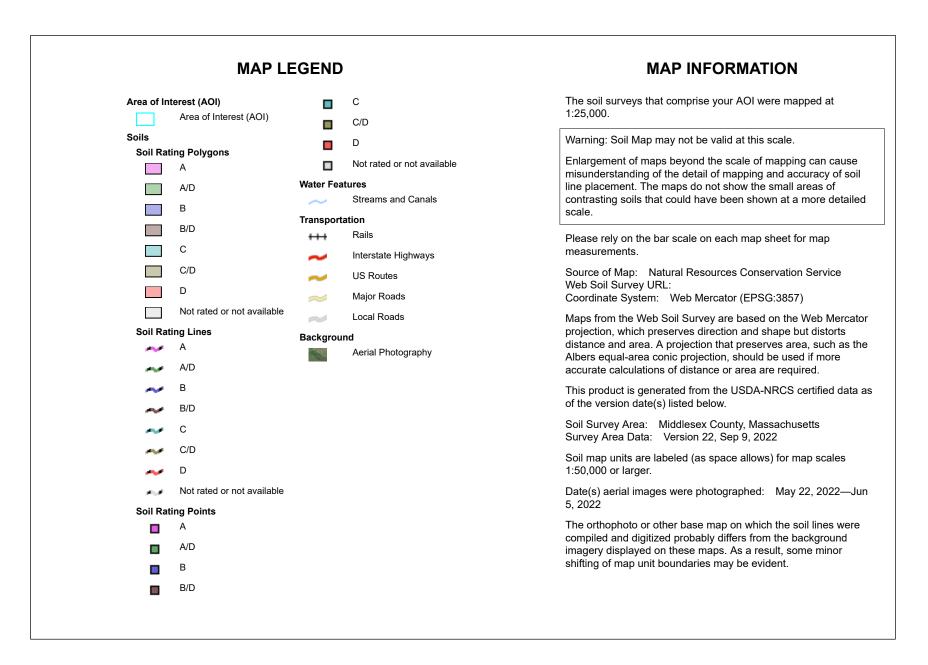


## **APPENDIX B. SOIL INFORMATION**

NRCS Soils Report (from NRCS Website)



USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey





# Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
6A	Scarboro mucky fine sandy loam, 0 to 3 percent slopes	A/D	0.0	0.0%
253A	Hinckley loamy sand, 0 to 3 percent slopes	A	19.0	23.8%
255A	Windsor loamy sand, 0 to 3 percent slopes	A	36.0	45.3%
256A	Deerfield loamy fine sand, 0 to 3 percent slopes	A	24.6	30.9%
Totals for Area of Interest			79.6	100.0%

# Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

# **Rating Options**

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher



Deep Observation Hole Logs



### **DEEP OBSERVATION HOLE LOGS**

DATE:	<u>May 10, 2024</u>	JOB NO:	<u>22-243</u>
LOCATION:	<u>63 Gratuity Road</u> Groton, MA	PREPARED FOR:	Routhier & Roper Gratuity Road, LLC
ATTENDEES:	Matthew Stangle, LandTech	APPROVING AUTHORITY:	<u>Unofficial</u> (Stormwater)

Number	Depth (inches)	Soil Horizon	Soil Color	Soil Description
TH-101	0-12	А	10Y/R 3/2	Sandy Loam
	12-16	В	10Y/R 6/8	Loamy Sand
	16-72	С	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	Water @ 52"	ESHWT @ 42"	
TH-102	0-8	A	10Y/R 3/2	Sandy Loam
111 102	8-18	B	101/R 5/2 10Y/R 6/8	Loamy Sand
	18-60	C	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	Water @ 48"	ESHWT @ 40"	
TH-103	0-8	А	10Y/R 3/2	Sandy Loam
111-105	8-16	B	101/R 5/2 10Y/R 6/8	Loamy Sand
	16-66	C	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	Water @ 52"	ESHWT @ 36"	
TH-104	0-8	А	10Y/R 3/2	Sandy Loam
111-104	8-16	B	101/R 5/2 10Y/R 6/8	Loamy Sand
	16-60	C C	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	Water @ 36"	ESHWT @ 32"	



Number	Depth (inches)	Soil Horizon	Soil Color	Soil Description
TH-105	0-12	А	10Y/R 3/2	Sandy Loam
	12-16	В	10Y/R 6/8	Loamy Sand
	16-48	С	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	Water @ 32"	ESHWT @ 20"	
TH-106	0-12	A	10Y/R 3/2	Sandy Loam
111 100	12-18	B	10Y/R 6/8	Loamy Sand
	18-48	Ċ	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	Water @ 36"	ESHWT @ 24"	
TH-107	0-8	A	10Y/R 3/2	Sandy Loom
111-107	8-16	B	101/R 5/2 10Y/R 6/8	Sandy Loam Loamy Sand
	16-54	В С	2.5Y5/2	Medium-Course Sand
	10-04	C	2.313/2	wiculum-Course Sand
Notes:	No Refusal	Water @ 32"	ESHWT @ 20"	



### **DEEP OBSERVATION HOLE LOGS**

DATE:	December 10, 2024	<b>JOB NO:</b>	<u>22-243</u>
LOCATION:	<u>63 Gratuity Road</u> Groton, MA	PREPARED FOR:	Routhier & Roper Gratuity Road, LLC
ATTENDEES:	Matthew Stangle, LandTech	APPROVING AUTHORITY:	<u>Unofficial</u> (Stormwater)

Number	Depth (inches)	Soil Horizon	Soil Color	Soil Description
TH-201	0-8	А	10Y/R 3/2	Sandy Loam
	8-18	В	10Y/R 6/8	Loamy Sand
	18-72	С	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	No Water	ESHWT @ 36"	
TH-202	0-12	A	10Y/R 3/2	Sandy Loam
	12-18	B	101/R 5/2 10Y/R 6/8	Loamy Sand
	18-90	C	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	Water @ 72"	ESHWT @ 42"	
TH-203	0-12	А	10Y/R 3/2	Sandy Loom
111-203	12-24	B	101/R 5/2 10Y/R 6/8	Sandy Loam Loamy Sand
	24-80	C B	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	No Water	ESHWT @ 24"	
TH-204	0-12	А	10Y/R 3/2	Sandy Loam
111 201	12-24	B	101/R 5/2 10Y/R 6/8	Loamy Sand
	24-78	C C	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	No Water	ESHWT @ 20"	



Number	Depth (inches)	Soil Horizon	Soil Color	Soil Description
TH-205	0-12	А	10Y/R 3/2	Sandy Loam
	12-20	B	10Y/R 6/8	Loamy Sand
	20-72	С	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	No Water	ESHWT @ 20"	



### **DEEP OBSERVATION HOLE LOGS**

DATE:	January 22, 2025	JOB NO:	<u>22-243</u>
LOCATION:	<u>63 Gratuity Road</u> Groton, MA	PREPARED FOR:	Routhier & Roper Gratuity Road, LLC
ATTENDEES:	Matthew Stangle, LandTech	APPROVING AUTHORITY:	<u>Unofficial</u> (Stormwater)

Number	Depth (inches)	Soil Horizon	Soil Color	Soil Description
TH-206	0-12	А	10Y/R 3/2	Sandy Loam
	12-21	В	10Y/R 6/8	Loamy Sand
	21-84	С	2.5Y5/2	Medium-Course Sand
Notes:	No Refusal	Weep @ 60"	Water @ 64"	ESHWT @ 24"
TH-207	0-8	А	10Y/R 3/2	Sandy Loam
	8-18	В	10Y/R 6/8	Loamy Sand
	18-78	С	2.5Y5/2	Medium Sand
Notes:	No Refusal	Weep/Water @ 66"	ESHWT @ 24"	
TH-208	0-12	А	10Y/R 3/2	Sandy Loam
	12-18	В	10Y/R 6/8	Loamy Sand
	24-72	Ċ	2.5Y5/2	Medium Sand
Notes:	No Refusal	Water @ 72"	ESHWT @ 21"	



# APPENDIX C. DEP STORMWATER MANAGEMENT CHECKLIST



# Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program Checklist for Stormwater Report

# A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.<sup>1</sup> This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8<sup>2</sup>
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

<sup>&</sup>lt;sup>1</sup> The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

<sup>&</sup>lt;sup>2</sup> For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



# **B. Stormwater Checklist and Certification**

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

*Note:* Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

## **Registered Professional Engineer's Certification**

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

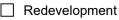
MATTHEW A. WATERMAN CIVIL No. 55666 Signature and Date

Registered Professional Engineer Block and Signature

## Checklist

**Project Type:** Is the application for new development, redevelopment, or a mix of new and redevelopment?

New development



Mix of New Development and Redevelopment



**LID Measures:** Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

$\boxtimes$	No disturbance to any Wetland Resource Areas	

- Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- Reduced Impervious Area (Redevelopment Only)
- Minimizing disturbance to existing trees and shrubs
- LID Site Design Credit Requested:
  - Credit 1
  - Credit 2
  - Credit 3
- Use of "country drainage" versus curb and gutter conveyance and pipe
- Bioretention Cells (includes Rain Gardens)
- Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- Treebox Filter
- Water Quality Swale
- Grass Channel
- Green Roof
- Other (describe):

#### **Standard 1: No New Untreated Discharges**

- No new untreated discharges
- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



#### Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

Calculations provided to show that post-development peak discharge rates do not exceed predevelopment rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24hour storm.

#### Standard 3: Recharge

Soil Analysis provided.

- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.

$\boxtimes$	Static
-------------	--------

Dynamic Field<sup>1</sup>

Runoff from all impervious areas at the site discharging to the infiltration BMP.

Simple Dynamic

Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.

Recharge BMPs have been sized to infiltrate	the Required Recharge Volume.
---------------------------------------------	-------------------------------

Recharge BMPs have been sized to infiltrate the Required Recharge Volume only to the maximum
extent practicable for the following reason:

- Site is comprised solely of C and D soils and/or bedrock at the land surface
- M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
- Solid Waste Landfill pursuant to 310 CMR 19.000
- Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

	Property include	es a M.G.L. c. 2′	E site or a solid	l waste landfill a	and a mounding	analysis is included.
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<sup>&</sup>lt;sup>1</sup> 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



#### Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

#### **Standard 4: Water Quality**

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- · Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
- Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
  - is within the Zone II or Interim Wellhead Protection Area
  - is near or to other critical areas
  - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
  - involves runoff from land uses with higher potential pollutant loads.
- The Required Water Quality Volume is reduced through use of the LID site Design Credits.
- Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



# Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program Checklist for Stormwater Report

Standard 4: Water Quality (continued)								
The BMP is sized (and calculations provided) based on:								
$\boxtimes$ The ½" or 1" Water Quality Volume or								
The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.								
The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.								
A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.								
Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)								
<ul> <li>The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.</li> <li>The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted <i>prior to</i> the discharge of stormwater to the post-construction stormwater BMPs.</li> </ul>								
The NPDES Multi-Sector General Permit does <i>not</i> cover the land use.								
☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.								
All exposure has been eliminated.								
All exposure has <i>not</i> been eliminated and all BMPs selected are on MassDEP LUHPPL list.								
The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.								
Standard 6: Critical Areas								
The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.								
Critical areas and BMPs are identified in the Stormwater Report.								



# Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

- Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- Bike Path and/or Foot Path
- Redevelopment Project
- Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

#### Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



# **Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control** (continued)

- ☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has *not* been included in the Stormwater Report but will be submitted *before* land disturbance begins.
- The project is *not* covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

#### **Standard 9: Operation and Maintenance Plan**

- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
  - Name of the stormwater management system owners;
  - Party responsible for operation and maintenance;
  - Schedule for implementation of routine and non-routine maintenance tasks;
  - Plan showing the location of all stormwater BMPs maintenance access areas;
  - Description and delineation of public safety features;
  - Estimated operation and maintenance budget; and
  - Operation and Maintenance Log Form.
- The responsible party is *not* the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
  - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
  - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

#### Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted *prior to* the discharge of any stormwater to post-construction BMPs.



# APPENDIX D. OPERATION AND MAINTENANCE LOG

Operation and Maintenance Log Page **1** of **3** 

Inspections for Year: \_\_\_\_\_

NOTE: See Standard 9 Operations and Maintenance of the Stormwater Management Report for additional details.

Best Management Practice	Action	Date Completed	Comments	Completed By	Action	Date Completed	Comments	Completed By
Deep Sump Catch Basins - Inspect or	Inspect				Inspect			
clean deep sump basins at least four times per year and at the end of the	Inspect				Inspect			
foliage and snow removal seasons.	Inspect				Inspect			
Sediments must also be removed four times per year or whenever the depth of	Inspect				Inspect			
deposits is greater than or equal to one half the depth from the bottom of the	Inspect				Inspect			
invert of the lowest pipe in the basin.	Inspect				Inspect			
	Inspect				Inspect			
	Inspect				Inspect			
	Clean				Clean			
	Clean				Clean			
Sediment Fersher	Inspect				Inspect			
Sediment Forebay	Inspect				Inspect			
Visually inspect monthly during first year of operation, after which visually inspect	Inspect				Inspect			
every 3 months at a minimum. Clean	Inspect				Inspect			
when average depth of sediment exceeds three inches. Mow grasses	Inspect				Inspect			
when exceeding 6 inches and replace vegetation damaged during cleaning.	Inspect				Inspect			
	Clean				Clean			
	Clean				Clean			

Best Management Practice	Action	Date Completed	Comments	Completed By	Action	Date Completed	Comments	Completed By
	Inspect				Inspect			
Pretreatment Structures (Turret) – Inspect or clean drain separator	Inspect				Inspect			
structures similar to catch basins, at least	Inspect				Inspect			
four times per year. Sediments must be removed whenever the depth of deposits is greater than or equal to one half the	Inspect				Inspect			
depth from bottom of sump to outlet	Inspect				Inspect			
elevation. Sediment to be removed from collection chamber with shovels or with	Inspect				Inspect			
hydro-vac, and drop in filters to be cleaned with broom or hose.	Clean				Clean			
cleaned with broom of hose.	Clean				Clean			
Infiltration Basin – Visual inspection for proper functioning will occur after every	Inspect				Inspect			
major storm during the first three months	Inspect				Inspect			
of operation and twice a year thereafter. Visual inspection and preventative	Inspect				Inspect			
maintenance will occur at least twice per	Inspect				Inspect			
year, and after every time drainage discharges through emergency spillways.	Inspect				Inspect			
Mow the buffer area, side slopes, and basin bottom; remove trash and debris;	Inspect				Inspect			
remove grass clippings and accumulated organic matter twice per year.	Clean				Clean			
organic matter twice per year.	Clean				Clean			

Best Management Practice	Action	Date Completed	Comments	Completed By	Action	Date Completed	Comments	Completed By
	Inspect				Inspect			
<b>Contech CDS separator</b> – Inspection should be performed at least twice per	Inspect				Inspect			
year, though the frequency of	Inspect				Inspect			
maintenance may need to be increased or reduced based on local conditions.	Inspect				Inspect			
Maintenance shall be performed when the level of sediment has reached 75% of	Inspect				Inspect			
capacity in the isolated sump or when an	Inspect				Inspect			
appreciable level of hydrocarbons and trash has accumulated.	Clean				Clean			
	Clean				Clean			